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Stormwater Management Report

Proposed Woolworths Local Shopping Centre & 1 into 6 Lot Torrens Title Subdivision

Property:

309 George Booth Drive, Cameron Park
Lot 901 DP 1222132

Applicant:

Fabcot Pty Ltd

Date:

February 2018

Document Control Sheet

Issue No.	Amendment	Date	Prepared By	Checked By
A	Draft	20/06/17	Mark Littlefield	Mark Kelly
B	ESCP updates	20/02/18	Rexx Brown	Mark Kelly

Limitations Statement

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Unless otherwise specified in this report, information and advice received from external parties during the course of this project was not independently verified. However, any such information was, in our opinion, deemed to be current and relevant prior to its use. Whilst all reasonable skill, diligence and care have been taken to provide accurate information and appropriate recommendations, it is not warranted or guaranteed and no responsibility or liability for any information, opinion or commentary contained herein or for any consequences of its use will be accepted by ADW Johnson or by any person involved in the preparation of this assessment and report.

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Table of Contents

1.0	INTRODUCTION	1
2.0	SITE DESCRIPTION.....	2
3.0	PROPOSED DEVELOPMENT.....	4
4.0	REQUIREMENTS	5
4.1	COUNCIL REQUIREMENTS	5
4.2	SPECIFIC SITE REQUIREMENTS	5
5.0	STORMWATER MANAGEMENT STRATEGY	6
5.1	OVERALL SYSTEM	6
5.2	WATER QUANTITY - MODELLING, BASE PARAMETERS & CATCHMENTS.....	6
5.3	WATER QUALITY STRATEGY	8
5.4	SOIL & WATER MANAGEMENT	9
6.0	KEY COMPLIANCE	10
7.0	CONCLUSION.....	12

APPENDICES

- Appendix A** Stormwater Management Plan
- Appendix B** Concept Engineering Plans including Erosion and Sediment Control Plan
- Appendix C** Existing Site
- Appendix D** Sediment Basin Sizing Calculation
- Appendix E** Basin Maintenance Checklist
- Appendix F** Extract from Cameron Grove Plans, (BROWNS Consulting)

1.0 Introduction

ADW Johnson Pty Ltd has been commissioned by Fabcot Pty Ltd to prepare a Stormwater Management Plan (SWMP) to accompany a Development Application (DA) for a proposed Woolworths Local Shopping Centre development and one (1) into six (6) lot Torrens Title Subdivision at Lot 901 DP 1222132, 309 George Booth Drive, Cameron Park. The proposed development is to be known as Cameron Park Village.

2.0 Site Description

The subject site is located within the Cameron Grove Residential Estate at Cameron Park, approved under DA 2433/2004. This subdivision approval included creation of the subject site to facilitate commercial development.

The subject site has an area of 5.652ha and frontages to Portland Drive, Northridge Drive, Tramway Drive and George Booth Drive.

In accordance with the abovementioned subdivision approval, substantial works have been undertaken including site vegetation clearing, earthworks, servicing infrastructure, drainage controls and road construction. Construction of the major intersection on George Booth Drive is well advanced.

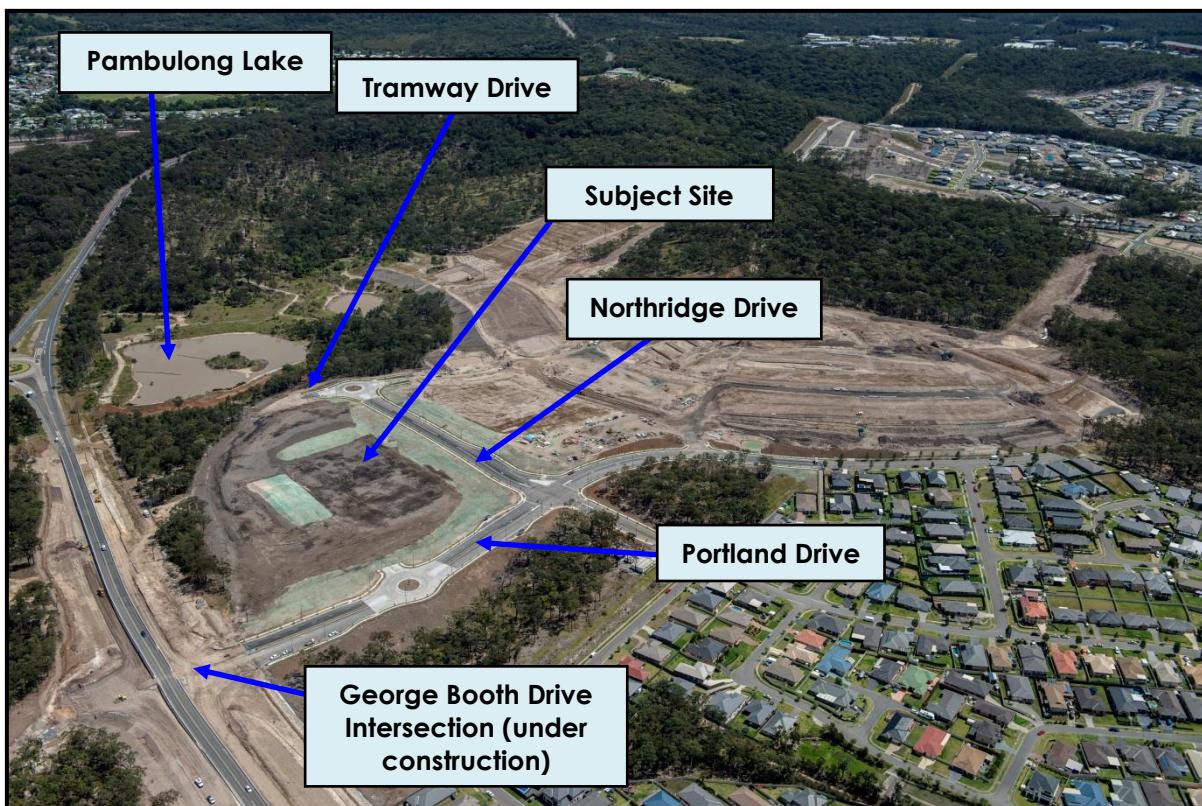


Figure 1: Aerial Image of Subject Site (October 2016)

Major stormwater infrastructure has already been constructed to serve the eastern portion of the site in accordance with the Cameron Grove subdivision approval (DA 2433/2004). Engineering plans for this existing infrastructure prepared by Brown Consulting are shown in **Appendix F**.

A stormwater and water control quality pond "Pambulong Lake" has been previously established to the west of the site (as seen above) and will provide the major storm water management including water quality and detention for the site in accordance with DA 2433/2004.

The subject site is not identified as containing potential contamination, and has been previously approved by Council to facilitate commercial use as part of the broader subdivision under DA 2433/2004.

The subject site is not identified on the Lake Macquarie LEP 2014 Acid Sulphate Soils map as containing potential Acid Sulphate Soils (ASS).

3.0 Proposed Development

The proponent is seeking to establish a local shopping centre with Woolworths as the anchor tenant. The development is to be known as Cameron Park Village.

Specific details of the proposal can be seen in the Statement of Environmental Effects prepared by ADW Johnson including architectural plans.

The figure below shows the proposed site layout.

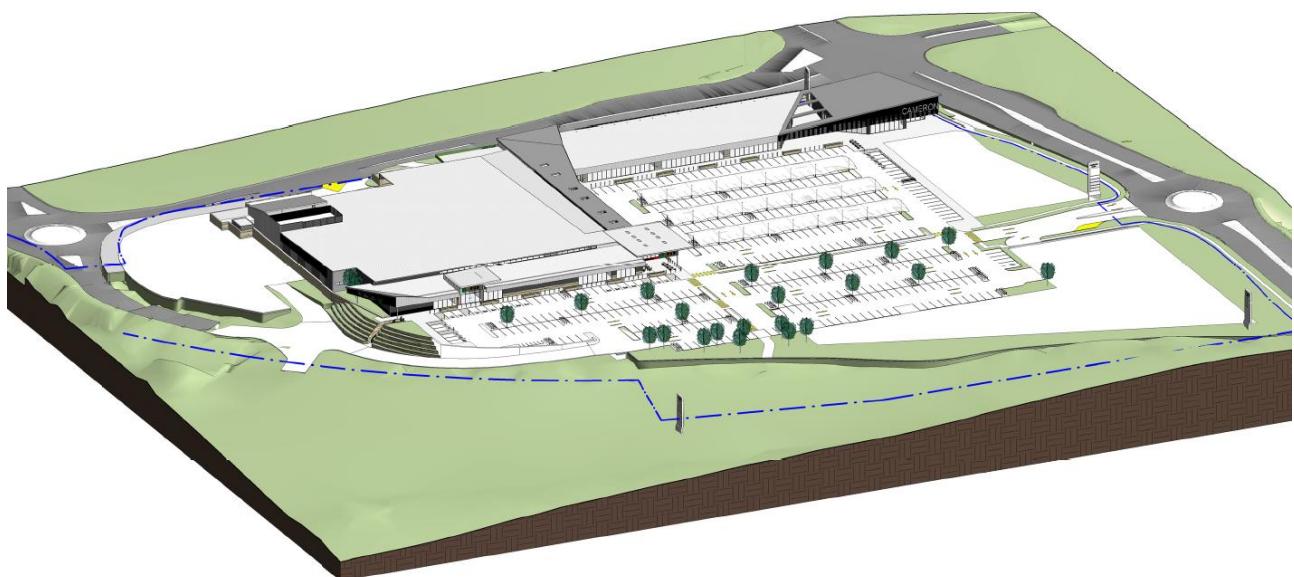


Figure 2: Indicative Proposed Development

4.0 Requirements

4.1 COUNCIL REQUIREMENTS

From Part 2.5 – Stormwater Management, Infrastructure and On-Site Services of Council's Development Control Plan No. 1 (DCP No. 1), the proposed development is described as that which: "*involve(s) water flows exceeding the equivalent water demand of 50 persons or more.*" This triggers the requirement for a comprehensive water cycle strategy to be prepared as an acceptable solution to mitigate potential stormwater impacts from the proposed development.

The stormwater management system is to minimise environmental impact of urban runoff. Typically, this is addressed by modelling simulation of water quality controls to target objectives. For this proposed development, the above has been addressed generally in the report prepared by Brown Consulting, "Stormwater Masterplan Lot 104, Pambulong Forest, Estelville" – January 2004. Additional controls above that specified in the report by Brown consulting are detailed within this report.

From Part 2.1.11 – Erosion Prevention & Sediment Control of Council's DCP No. 1, the proposed development is described as Category 3 – greater than 2,500m² (of disturbance). This triggers the requirement for a Soil and Water Management Plan (SWMP) to be prepared to mitigate potential stormwater impacts during construction of the proposed development. General requirements for the preparation of the SWMP are described by Council guidelines and the Soils and Construction – Volume 1, 4th Edition 'Blue Book' - (reprinted July 2006). Full details of the SWMP are described in **Section 5.4**.

4.2 SPECIFIC SITE REQUIREMENTS

Plans approved by Brown Consulting for Portland Drive (Project Ref L05016.014) have specified flows generated from the eastern portion of the site for both the minor (10 year) and major (100 year) events that are to be discharged through an adjoining development. It is therefore critical that these flows are not exceeded and detention measures provided if necessary.

5.0 Stormwater Management Strategy

The proposed strategy for the commercial development is in accordance with the approved strategy for the entire catchment. The strategy for water quality improvement utilises the existing "Pambulong Lake" for stormwater management for the western portion of the site, a smaller proposed basin for management of the eastern portion of the site and also includes end of line controls prior to discharge from the proposed development site in the form of GPT's. The overall strategy is shown in **Appendix A**.

5.1 OVERALL SYSTEM

There are five (5) controlled discharge points proposed from the site, with two (2) of these locations specified as significant points of discharge. These locations can be seen in Appendix A. The treatment elements selected for the improvement of stormwater quality prior to discharge from the proposed development site from the two (2) major discharge locations are:

- Gross Pollutant Traps (litter baskets may be substituted if accepted by Council);
- Rainwater tanks where appropriate; and
- "Pambulong Lake" (Western Portion of site), "Proposed Basin No.1" (Eastern Portion of site).

'Pambulong Lake' has been shown to provide adequate treatment for the western portion of the site. Downstream treatment measures have been designed and constructed for the eastern portion of the site in accordance with the report prepared by Brown Consulting, "Stormwater Masterplan Lot 104, Pambulong Forest, Estelville" – January 2004, which was approved as part of the Cameron Grove subdivision (DA 2433/2004). The proposed basin has been provided to ensure peak flow rates determined in the approved plans in both the minor and major storm are not exceeded. Although the basin has been placed to ensure peak flow rates are managed there will also be a treatment benefit in terms of reducing sediment, suspended solids and target nutrients.

Conventional piped street drainage will be used for the control of stormwater runoff from the roads and carpark. This system will convey the stormwater to the treatment elements outlined above.

5.2 WATER QUANTITY - MODELLING, BASE PARAMETERS & CATCHMENTS

The subcatchments were defined by concept design and site grading to distribute stormwater flows to the two (2) discharge locations described in **Section 5.1**. These catchments have been simplified into two (2) parts, namely Catchment East (Discharging to Portland Drive) and Catchment West (Discharging to "Pambulong Lake"). It has been determined that as flows for Catchment East connect to the existing drainage network on Portland Drive that the design flows approved under the Construction Certificate must not be exceeded by the current proposal. The discharge location for Catchment East is shown in Appendix A as well as in the figures below.

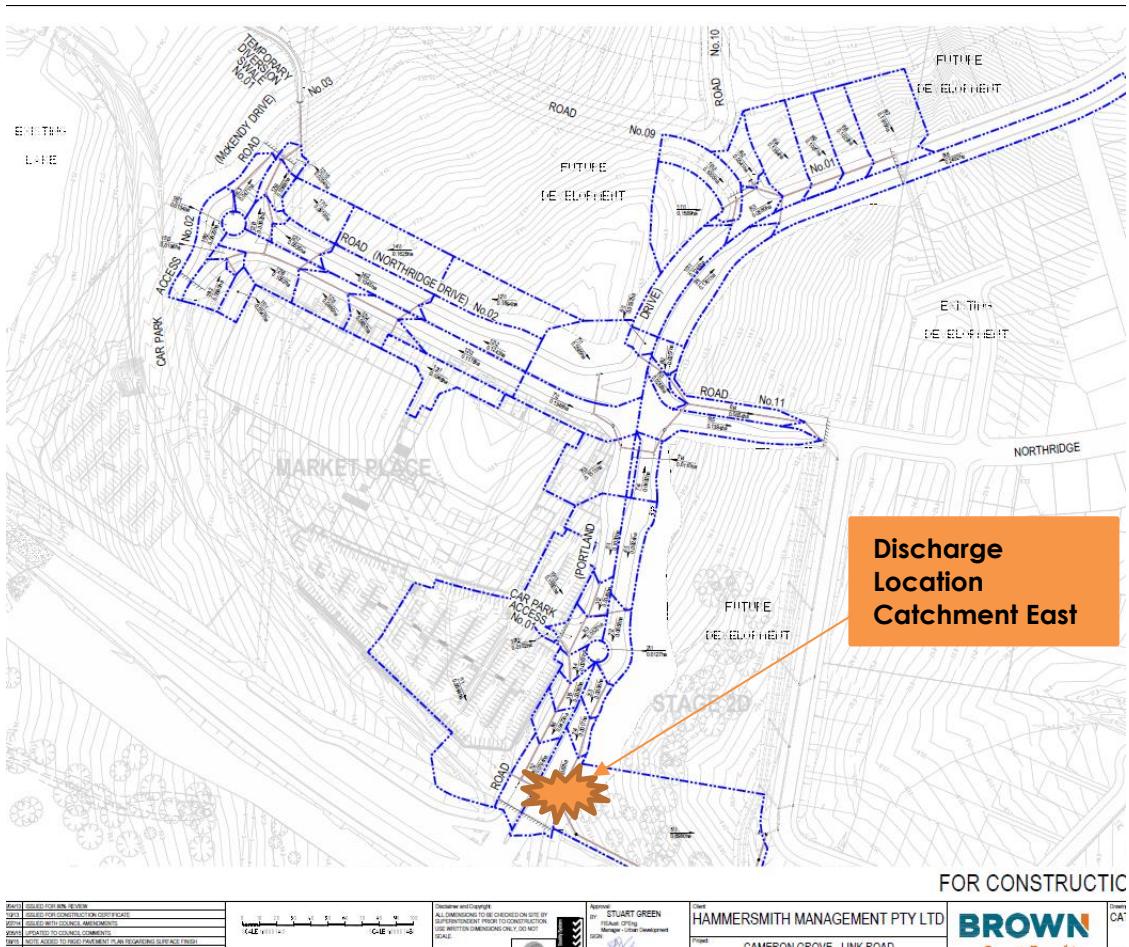


Figure 3: DA 2433/2004 Approved Catchment Plan (BROWN Consulting)

Based on the latest proposed Woolworths development the contributing catchment to Portland Drive has been modified and is shown below.

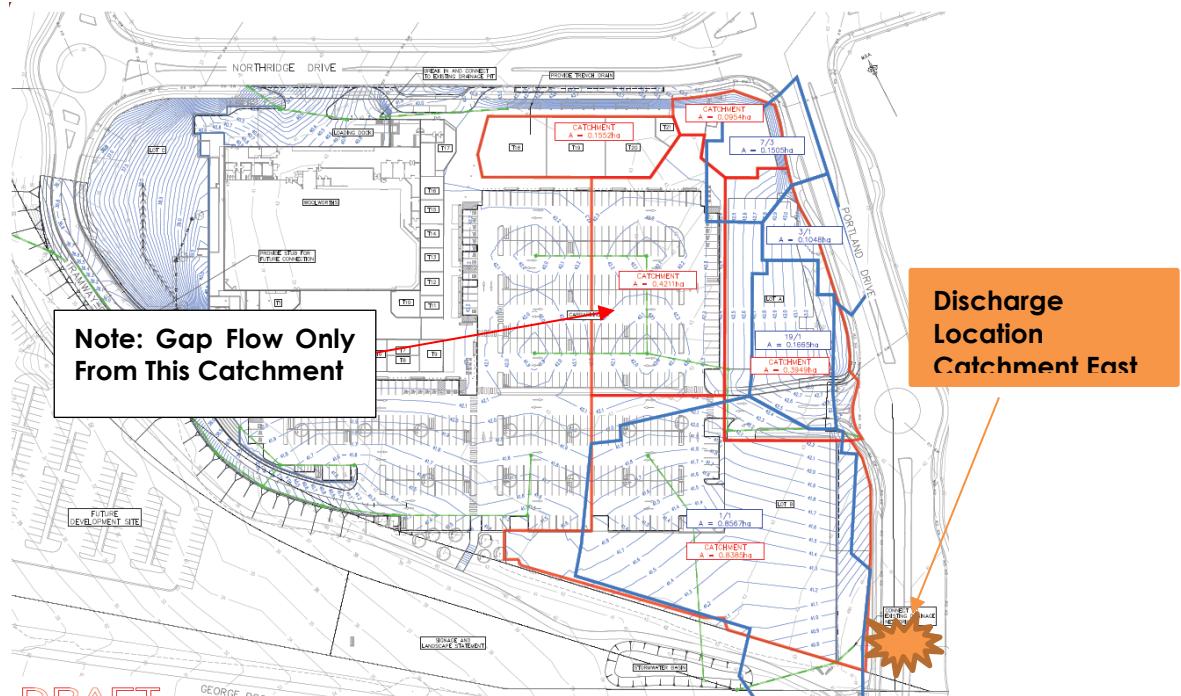


Figure 4: DA 2433/2004 Approved Catchments (Blue) overlaid with proposed catchments (Red)

To determine the peak design flows at the point of discharge a DRAINS storm water routing model was used. DRAINS uses the time-area method to estimate runoff hydrographs for pervious and impervious surfaces.

The Rainfall Intensity Frequency Duration (IFD) data used in the model was sourced from LMCC Handbook of Drainage Design Criteria.

Table 2 –Catchment East Analysis – Approved Catchment allowance off proposed site, Based off BROWNS plans

CATCHMENT	AREA (ha)	IMPERVIOUS (%)	PERVIOUS (%)	10 Yr ARI Peak Q (l/s)	100 Yr ARI Peak Q (l/s)
EAST A	1.01	80%	20%	401	578

Table 3 –Catchment East Analysis – Updated Catchment from proposed site

CATCHMENT	AREA (ha)	IMPERVIOUS (%)	PERVIOUS (%)	10 Yr ARI Peak Q (l/s)	100 Yr ARI Peak Q (l/s)
EAST B	1.63*	80%	20%	465	792*

*0.42ha of this is contributing gap flow only (Major storm flow – Minor storm flow)

An analysis of the peak flows from the site based on the original approved plans of the surrounding road network against the proposed development has determined that detention is now required to limit flows back to the peak flows approved.

To achieve this reduction in flows for the proposed development a detention basin has been modelled within DRAINS at the south end of the site. Details of the basin can be seen in **Appendix A**.

Table 4 – Catchment East with Detention

ARI Storm Event (yr)	Approved Development Peak Flow (l/s)	Post Development Peak Flow (l/s) with updated catchments	Post Development Peak Flow With Detention (l/s)
10	401	465	303
100	578	792	547

It can be seen that through the utilisation of the proposed detention basin flows generated from the development are less than that approved to be discharged within the plans prepared by Browns Consulting for Portland Drive.

5.3 WATER QUALITY STRATEGY

Gross Pollutant Traps (GPTs) are an essential element to the treatment train for water quality improvement. The traps are to collect heavy sediment and litter and provide a regulated location for maintenance, improving the ongoing function of the stormwater controls. There are two (2) GPT's proposed for the proposed development. The locations of the proposed GPT's can be seen in **Appendix A**.

A rainwater tank is proposed for the buildings. The rainwater tank will collect stormwater runoff from roof areas. The tank is to have appropriate first flush controls for maintenance purposes. Details for these controls will be specified with detailed design for the Construction Certificate.

The stormwater captured in the rainwater tanks will be used for toilet flushing and irrigation of landscaped areas. Harvesting rainwater tank water is a significant benefit in potable mains water reduction to the proposed development. Refer to **Appendix A** for rainwater tank locations.

The existing "Pambulong Lake" will provide effective detention for peak flows and retention of nutrients for the western portion of the site as well as be aesthetically pleasing. Details can be seen in the report "Stormwater Master plan, Lot 104 Pambulong Forest, Estelville" prepared by Brown Consulting (Refer Appendix F).

The proposed detention basin located at the south eastern corner of the site is included as a means to reduce peak flows but will also have an additional benefit of retaining nutrients before overflows are discharged through the catchment wide WSUD strategy described in the report "Stormwater Master Plan, Lot 104 Pambulong Forest, Estelville" prepared by Brown Consulting.

5.4 SOIL & WATER MANAGEMENT

A concept Soil and Water Management Plan has been prepared. This plan is to outline the temporary control measures for the protection of the downstream receiving waters during construction. Sediment fencing will be installed to boundaries of work areas. Stockpiles will be protected by sediment fencing. These measures need to be installed prior to commencement of earthworks. All controls need to be designed in accordance with 'Managing Urban Stormwater – Soils & Construction Volume 1' (Landcom, 2004) – 'Blue Book'. Refer to **Appendix B**.

Correspondence from LMCC has indicated the soils encountered are likely to be of type F/D which include dispersible particles. Standard monitoring techniques for Total Suspended Solids will be required during construction however the sediment basins are likely to require aided flocculation prior to pump-out/discharge.

Sediment basin sizing has been undertaken in accordance with the 'Blue Book', allowing for full site disturbance. Preliminary sizing indicates that utilising the proposed basins as temporary sediment basins with the following minimum volumes will satisfy the guidelines:

- Sediment Basin 1 – 874m³;
- Sediment Basin 2 – 364m³.

Refer to **Appendix D** for temporary sediment basin sizing calculations and **Appendix A** for temporary sediment basin locations.

6.0 Key Compliance

The key compliance requirements for stormwater strategy including the comprehensive water cycle and soil and water management requirements are shown below in **Tables 6** and **7**.

Table 6 – Comprehensive Water Cycle Requirements

Issue Description	Section Reference/ Comment
Site conditions, catchment context and land capability	Section 2.0
Estimates of all water flows	Section 5.2
Objectives and strategies for improving water efficiency, water quality, discharge volumes, concentrations, flood protection and aquatic environments	Section 5.3 – GPT's, rainwater tanks for irrigation and utilisation of existing downstream basins and proposed basins
The provision of integrated water infrastructure systems	Section 5.3 – Stormwater harvesting for irrigation of landscaping areas and other uses is an integrated system
Proposed layout and street design measures to minimise disturbance to natural landscape features and incorporate stormwater source controls in street reserves	Section 5.3 – GPT are included as at source controls
Proposed landscape design measures to protect natural features, improve water quality, provide recreation opportunities and satisfy safety requirements	Section 5.3 – Stormwater controls will be integrated as landscape features and not pose a risk in terms of safety
Proposed landscape practices to retain or restore natural landscape features, manage and treat stormwater runoff and reduce demand for water, fertilisers and herbicides	Section 5.3 – A rainwater tank for irrigation of landscaping areas and other non-potable uses located on site will reduce potable water demand to the development.
Provision of water tanks, infiltration devices and other on-site stormwater infrastructure at the subdivision stage including aquifer storage and harvesting of stormwater	Section 5.3 – GPT's, rainwater tanks for irrigation and basins
Wastewater reuse	None specified
Erosion, sediment and pollution control maintenance, monitoring and performance evaluation	Section 5.5

Table 7 – Soil and Water Management Requirements - Category 3

Issue Description	Section Reference/ Comment
Locality Details	Section 2.0
North point and scale	Appendix A
Property boundaries and adjoining roads	Appendix A, B
Existing land contours	Appendix C
Location of existing trees and vegetation	Appendix C
Location of existing significant landscape features	None present
Existing watercourses and drains flowing through, or adjacent to, the site	“Pambulong lake”
Outline of proposed building/structures and disturbed areas	Appendix A, B
Proposed vehicular access	Appendix A, B
Extent of vegetation to be cleared	Landscape Plan
Extent of earthworks and limits of cut and fill	Appendix B
Location of proposed stockpiles	Appendix B
Location of proposed temporary and permanent site drainage	No temporary drains proposed, Permanent are shown Appendix A
Location of proposed temporary erosion prevention and sediment control measures	Appendix B
Location of temporary and permanent revegetation areas	See landscape plans
An explanation of any changes to the erosion prevention and sediment controls as the works proceed	No change required
Supplementary notes covering inspection and maintenance requirements	Appendix F
SWMP's shall include detailed calculations to determine the soil loss and the size of any sediment basins that may be required on the site.	Appendix E
The location of lots, public open space, stormwater drainage systems, schools, shopping centres/community centres – (if nearby)	Appendix A, B
The location of land designated or zoned for special uses	N/A
Location and diagrams of all erosion and sediment site controls used	Appendix B
Locations, calculations and engineering details of any sediment basins	Appendix B, E
Location and details of other stormwater management structures such as; constructed wetlands, gross pollutant traps, trash racks or separators.	Appendix A

7.0 Conclusion

The key compliance requirements for stormwater management and soil and water management for the proposed Woolworths Shopping Centre have been met. The permanent controls for the site include Gross Pollutant Traps at each major outlet, rainwater tanks for stormwater harvesting and irrigation of the landscaped areas, and a detention basin. Further treatment measures have been implemented further downstream including "Pambulong Lake" and several significant stormwater basins east of the site.

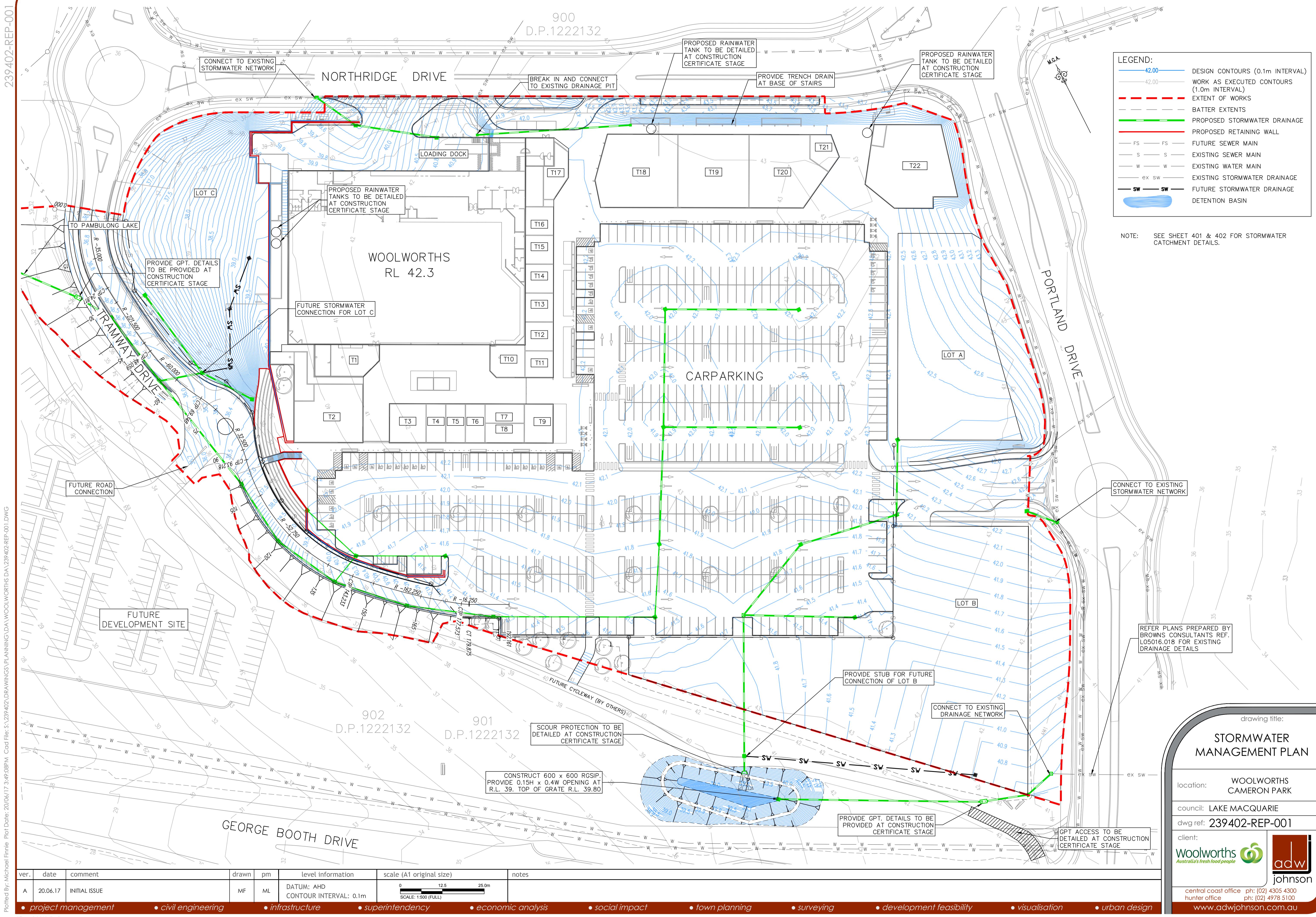
Peak flows being discharged through adjoining development east of the site has been modelled and the use of a detention basin has ensured approved flow rates from the site have not been exceeded.

The site is well elevated above regional 1:100 year ARI peak flood levels. Standard temporary sediment controls of sediment fencing will apply to the site during construction.



Appendix A

STORMWATER MANAGEMENT PLAN





Appendix B

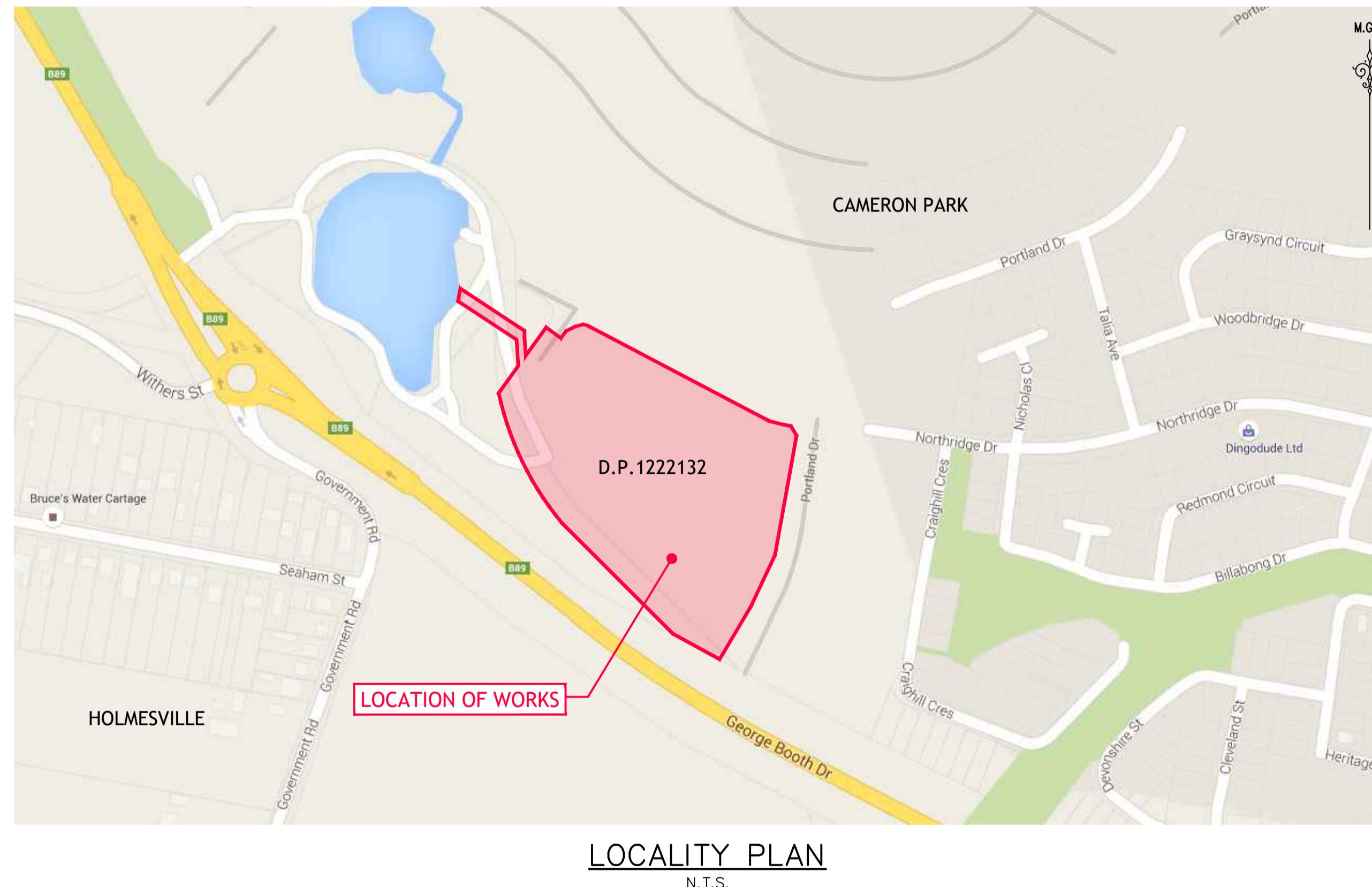
CONCEPT ENGINEERING INCLUDING EROSION AND SEDIMENT CONTROL PLAN



CAMERON PARK, LAKE MACQUARIE

D.P.1222132

DEVELOPMENT APPLICATION CIVIL DESIGN PLANS



INDEX OF DRAWINGS

DRAWING	NAME
239402-DA-001	TITLE SHEET, DRAWING INDEX AND LOCALITY PLAN
239402-DA-002	SITE LAYOUT PLAN
239402-DA-101	DETAIL PLAN - SHEET 1
239402-DA-102	DETAIL PLAN - SHEET 2
239402-DA-103	ACCESS ROAD LONGITUDINAL SECTION
239402-DA-201	EARTHWORKS PLAN
239402-DA-301	SITE SECTIONS - SHEET 1
239402-DA-302	SITE SECTIONS - SHEET 2
239402-DA-401	STORMWATER DRAINAGE PLAN - SHEET 1
239402-DA-402	STORMWATER DRAINAGE PLAN - SHEET 2
239402-DA-451	INDICATIVE ROOF CATCHMENT AND DOWNPipe PLAN
239402-DA-501	EROSION & SEDIMENT CONTROL PLAN - SHEET 1
239402-DA-502	EROSION & SEDIMENT CONTROL PLAN - SHEET 2
239402-DA-511	EROSION & SEDIMENT CONTROL DETAILS - SHEET 1
239402-DA-512	EROSION & SEDIMENT CONTROL DETAILS - SHEET 2
239402-DA-513	EROSION & SEDIMENT CONTROL NOTES
239402-DA-551	TYPICAL RETAINING WALL DETAILS & SECTIONS
239402-DA-601	SERVICES PLAN - SHEET 1
239402-DA-602	SERVICES PLAN - SHEET 2

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DRAWING INDEX &
LOCALITY PLAN**

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CAMERON PARK**

council: **LAKE MACQUARIE**

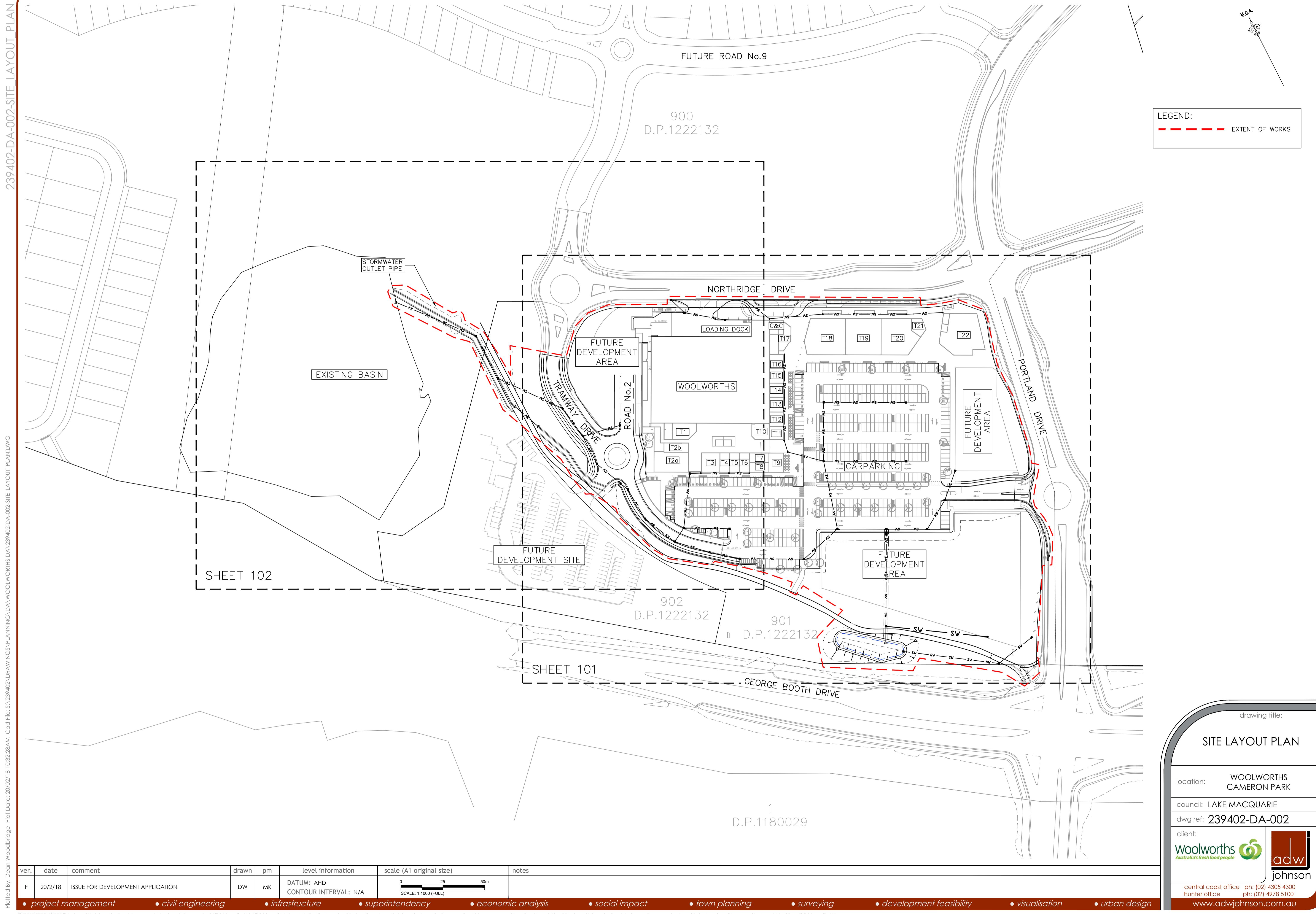
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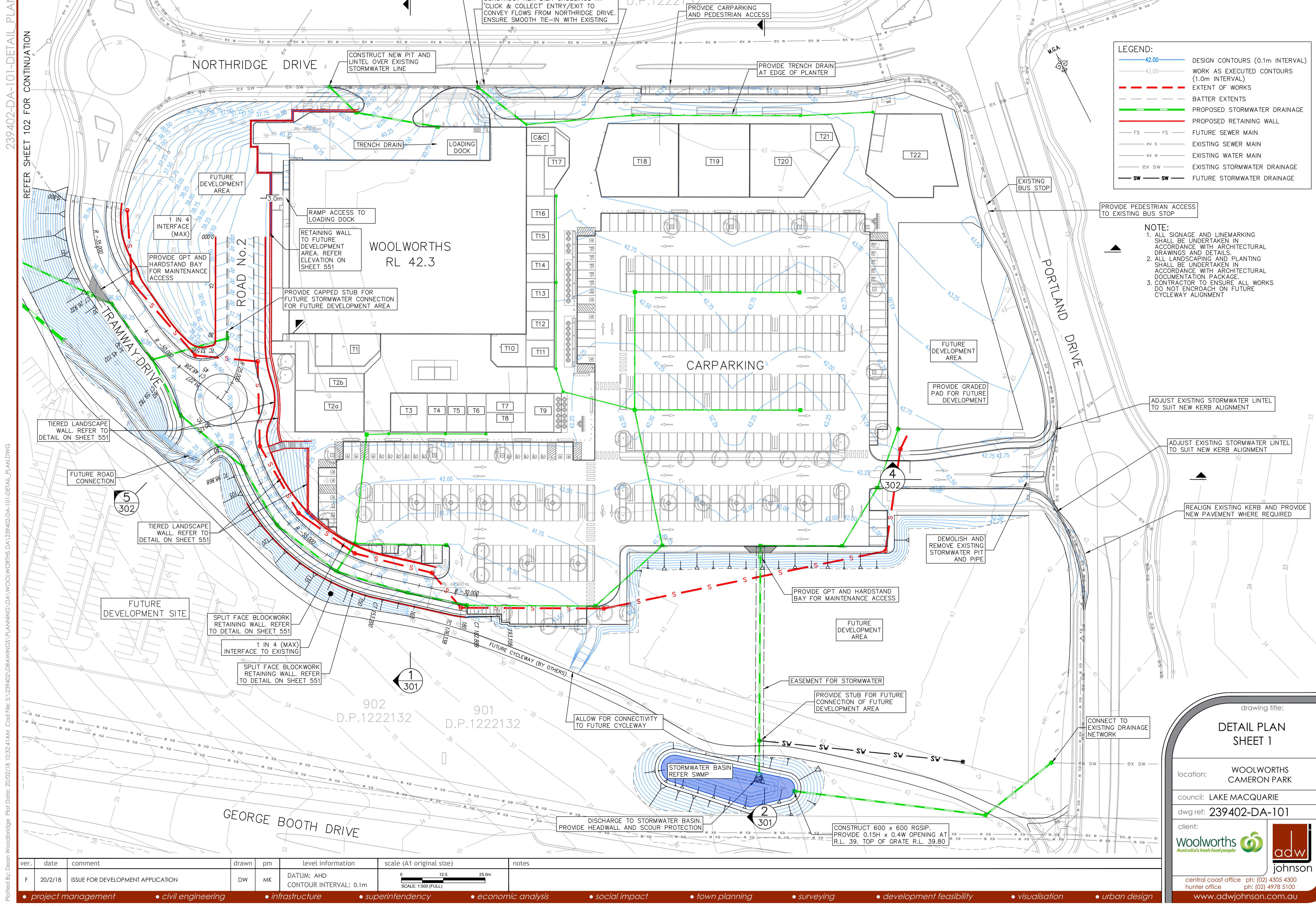
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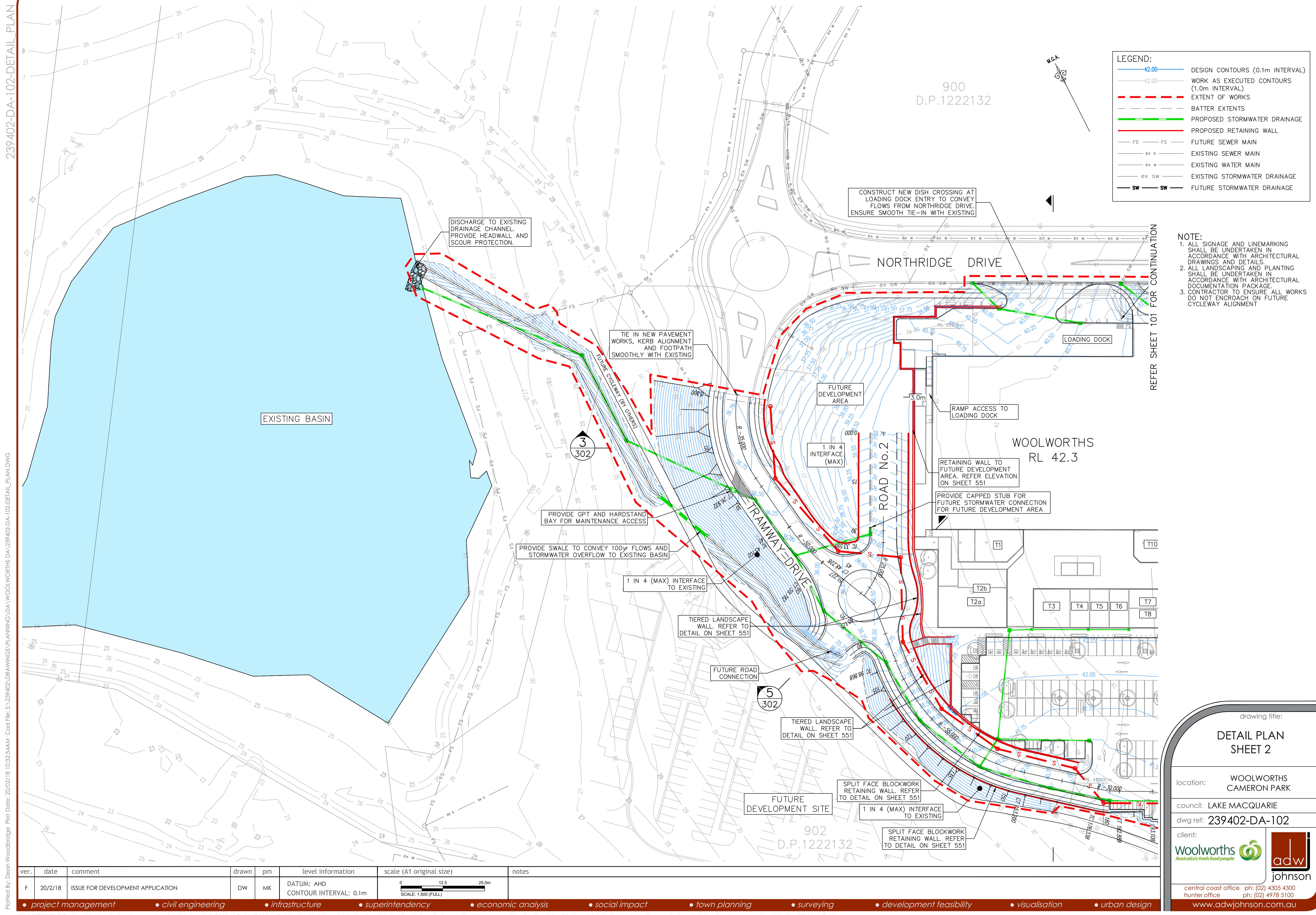
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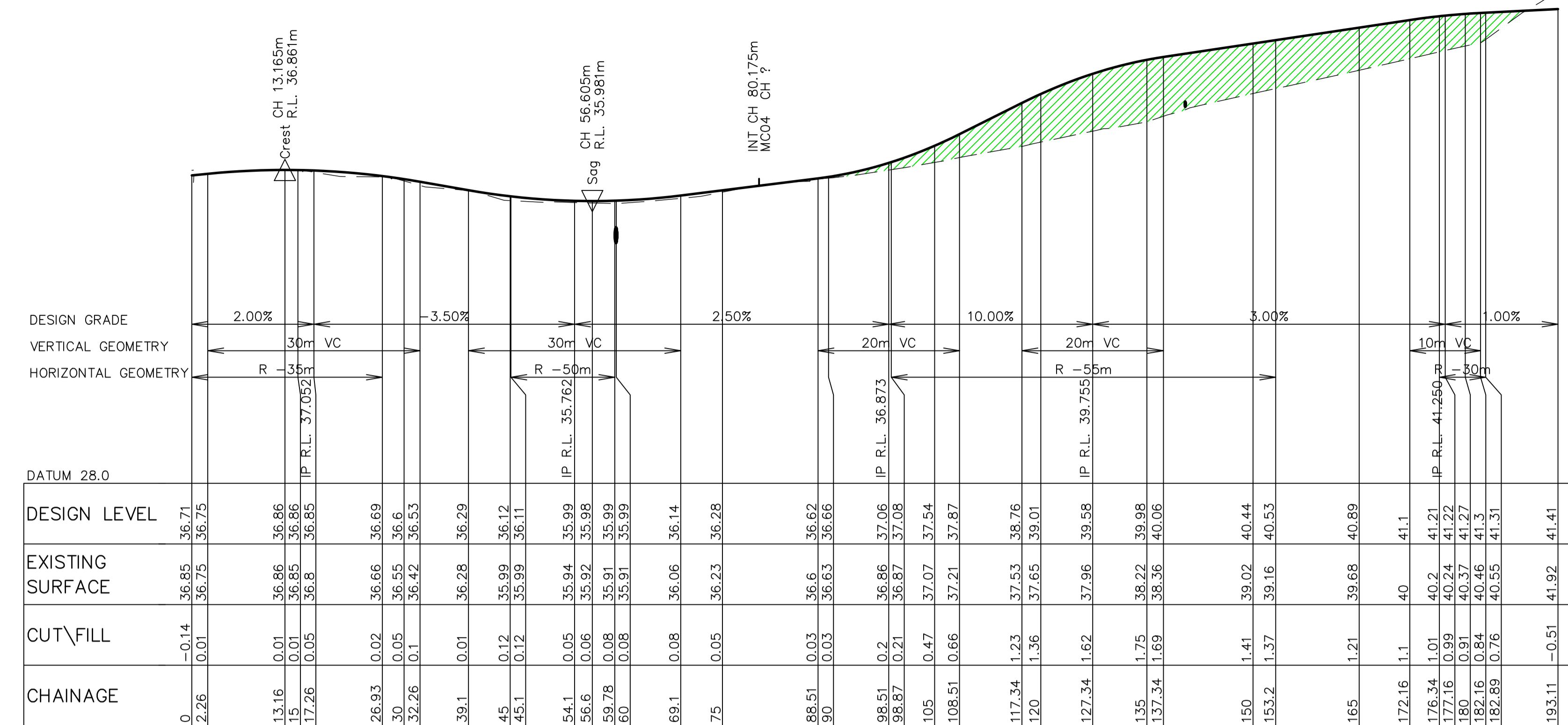
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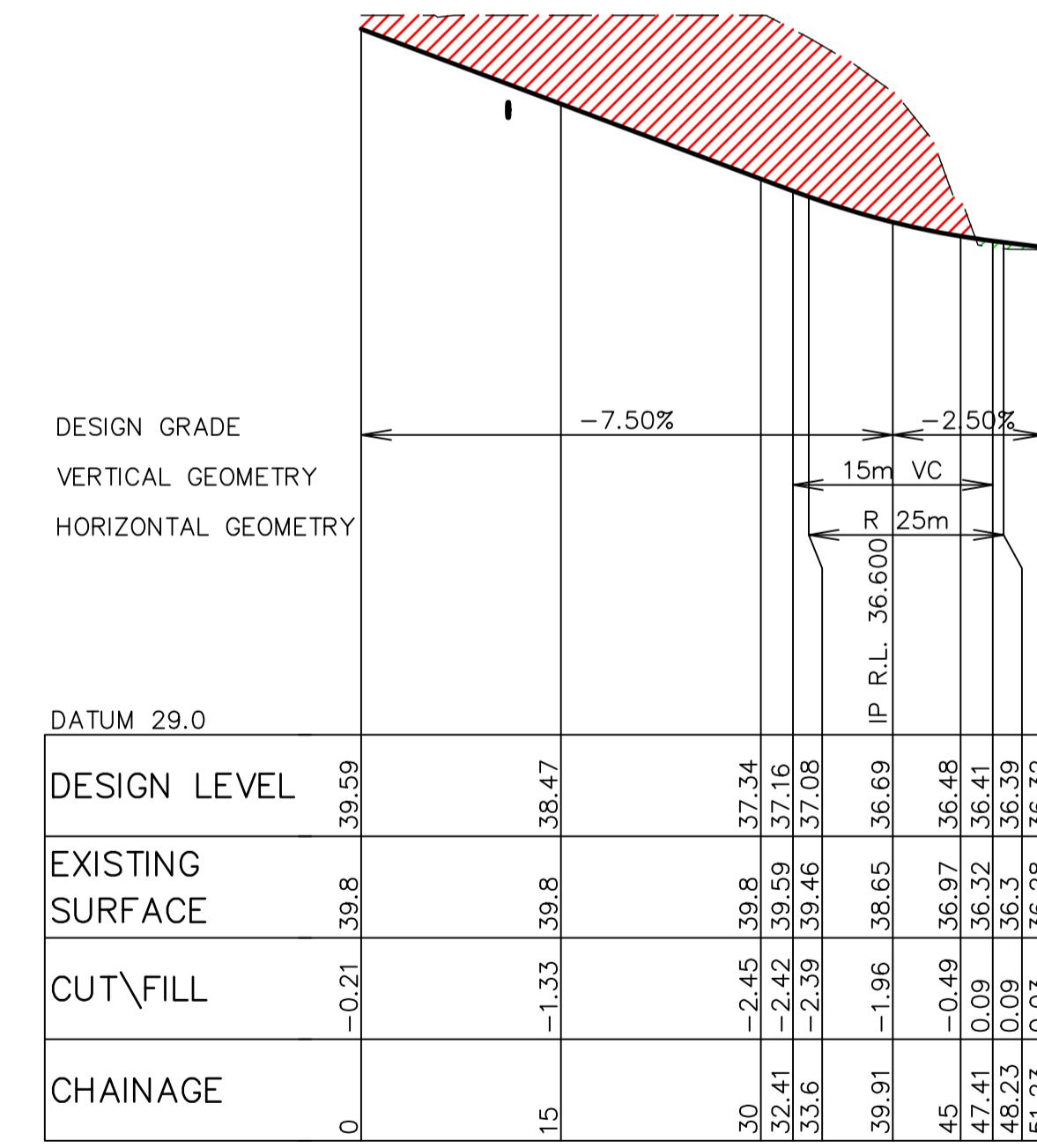
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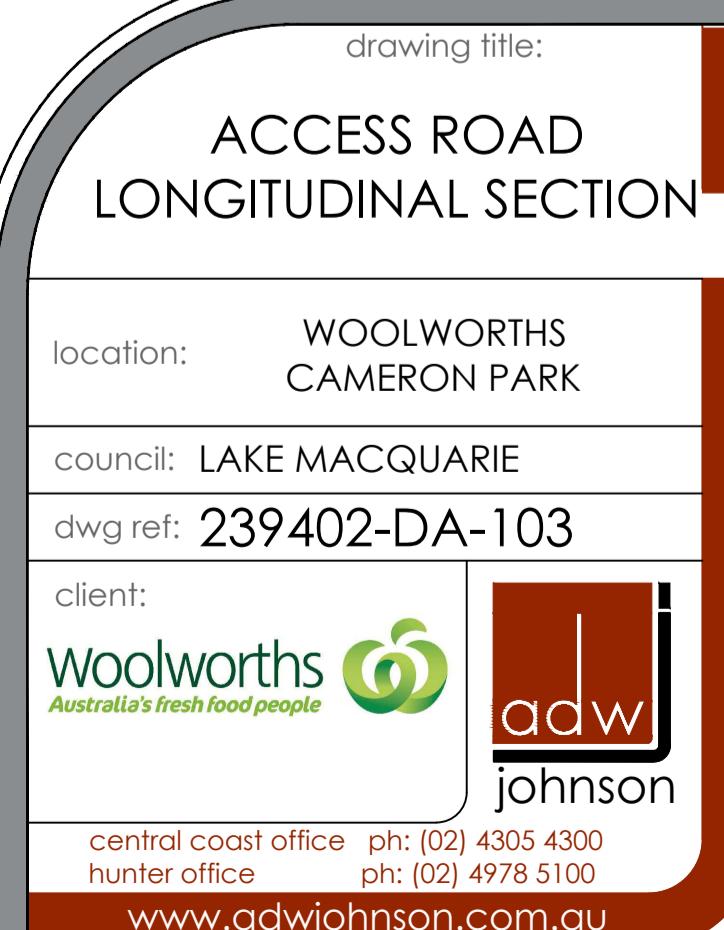
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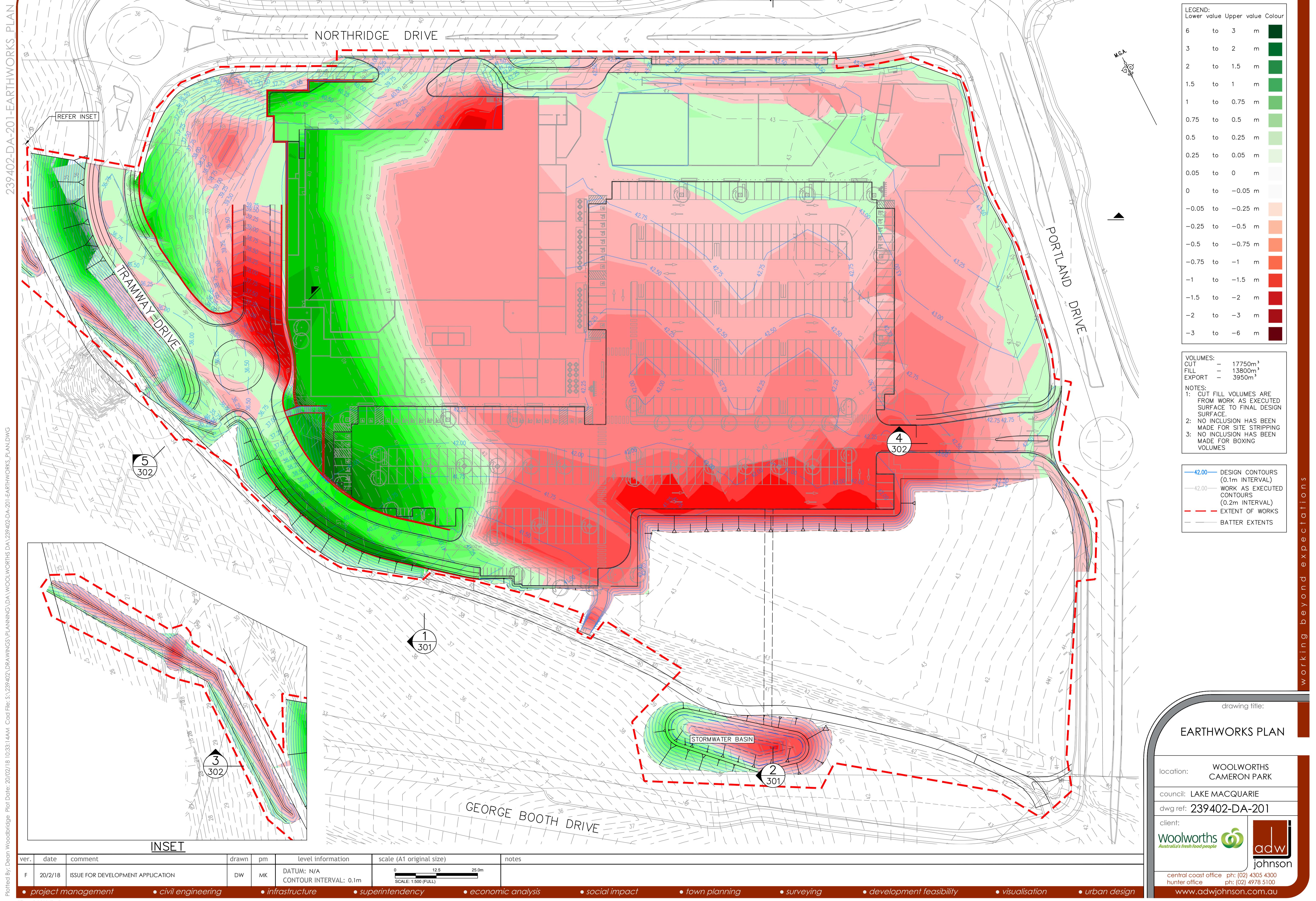
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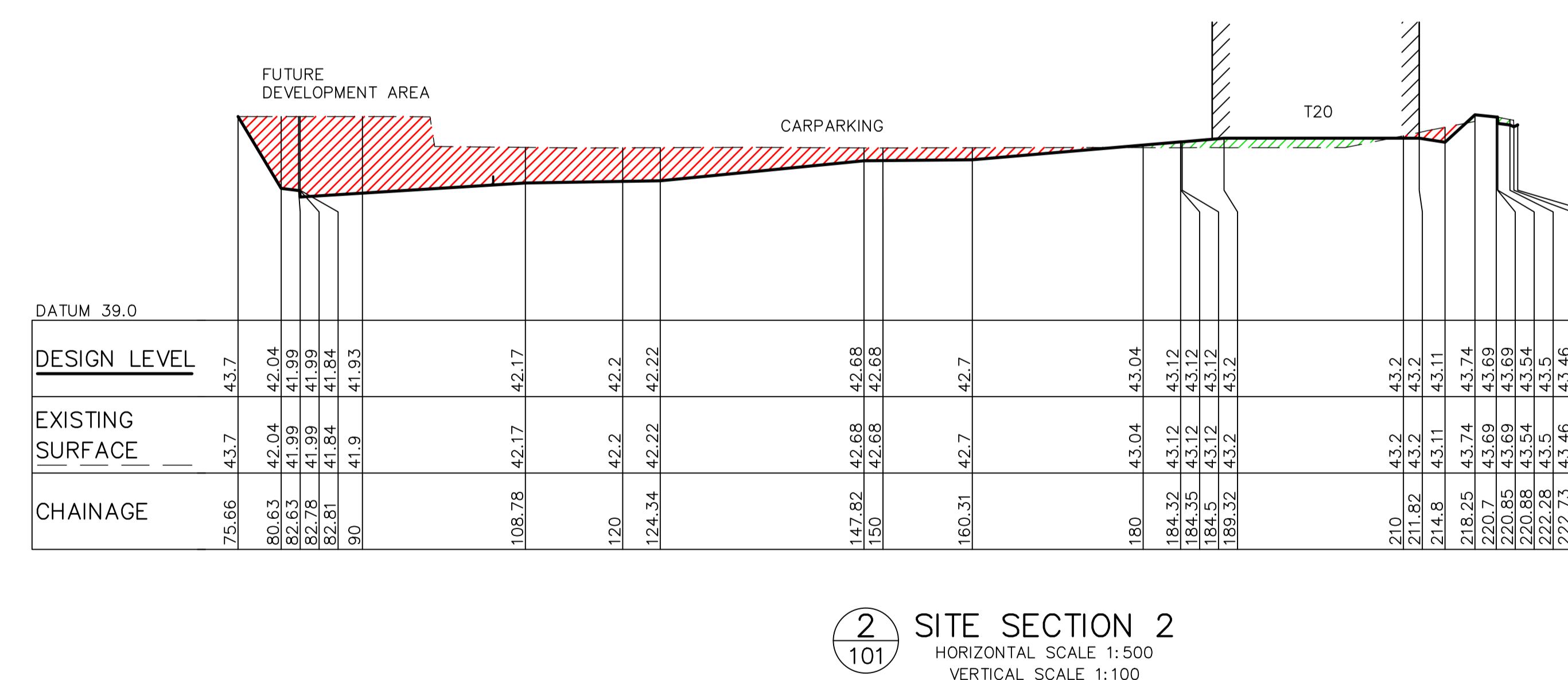
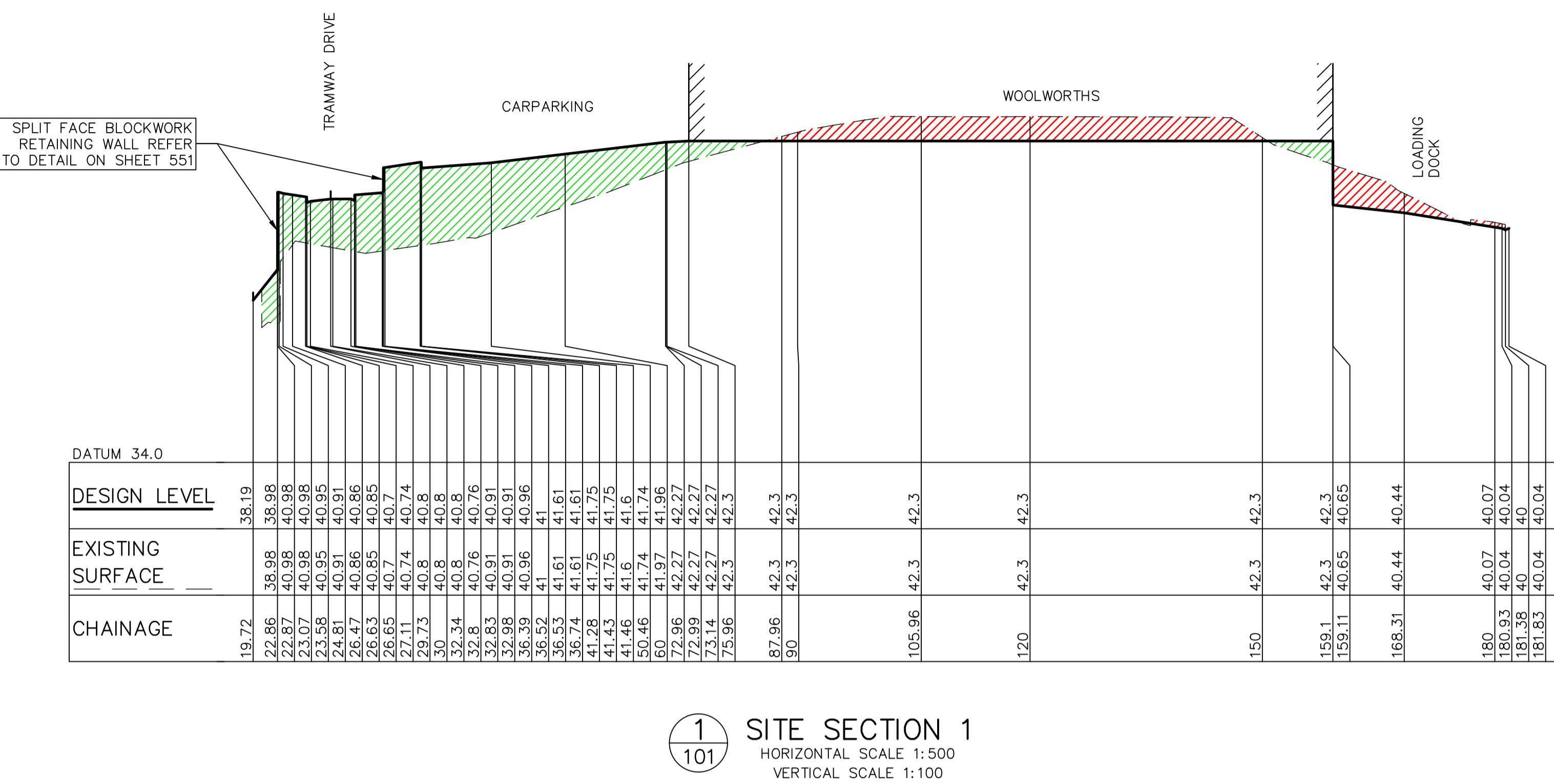
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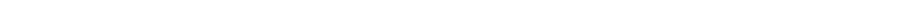
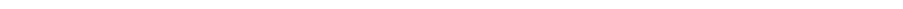
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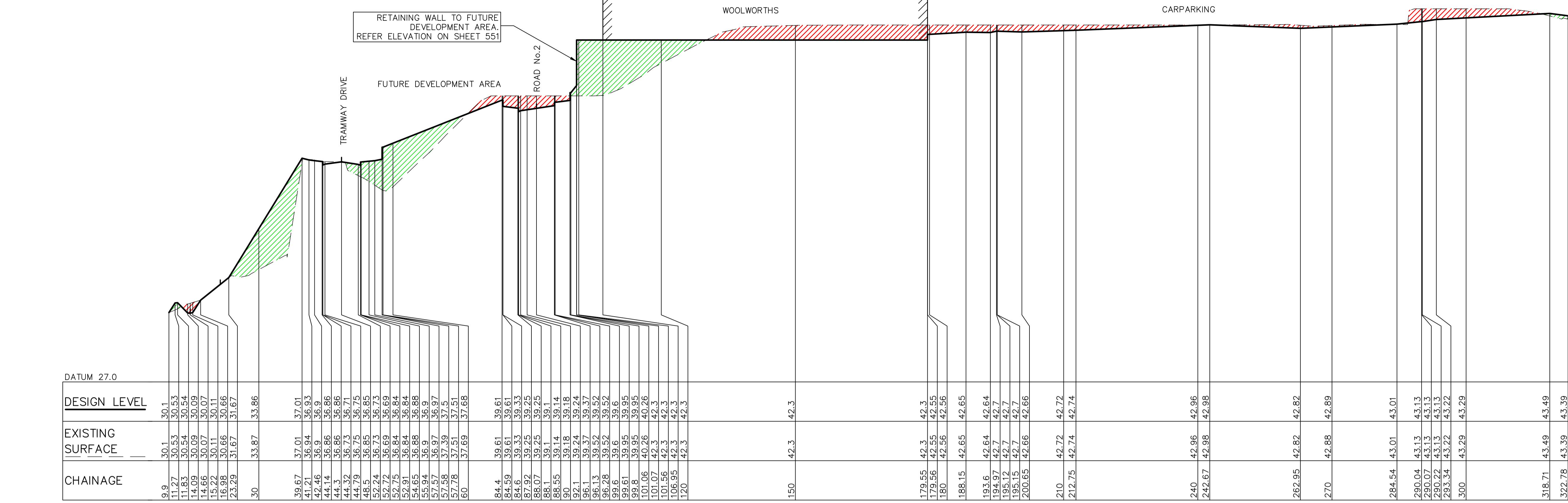
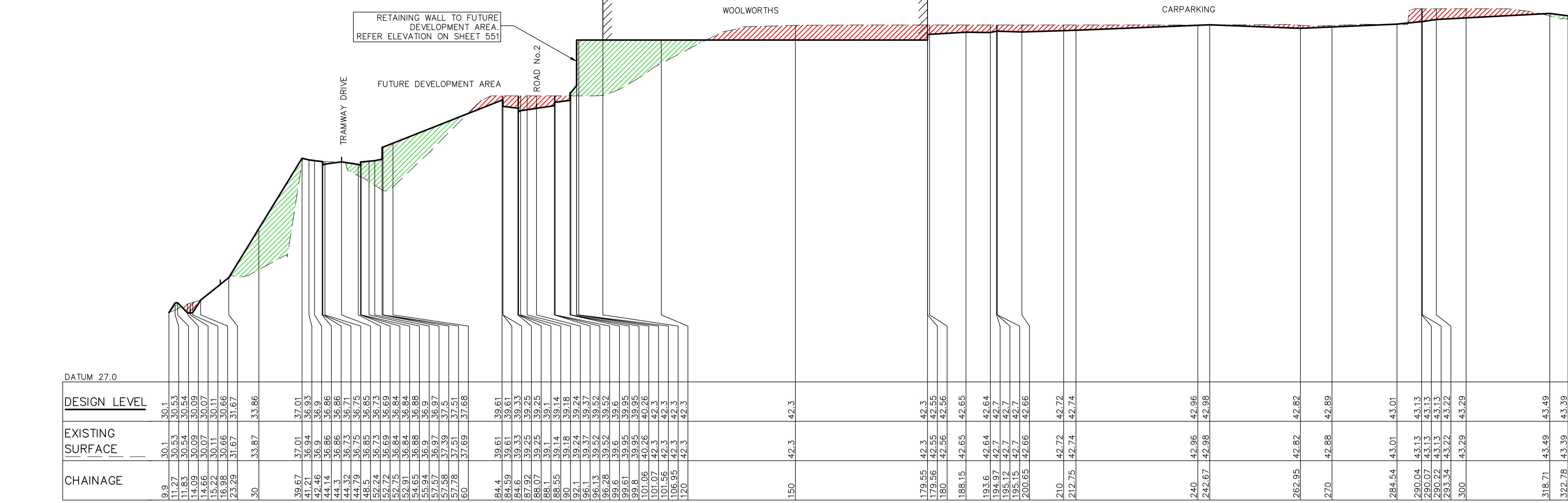
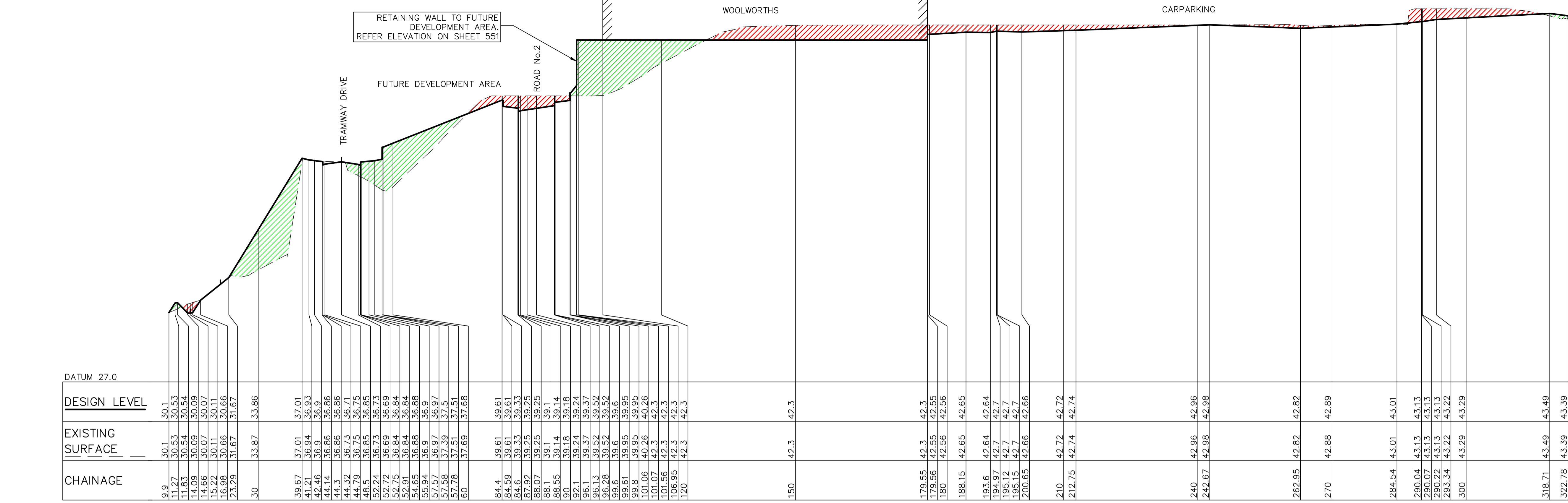
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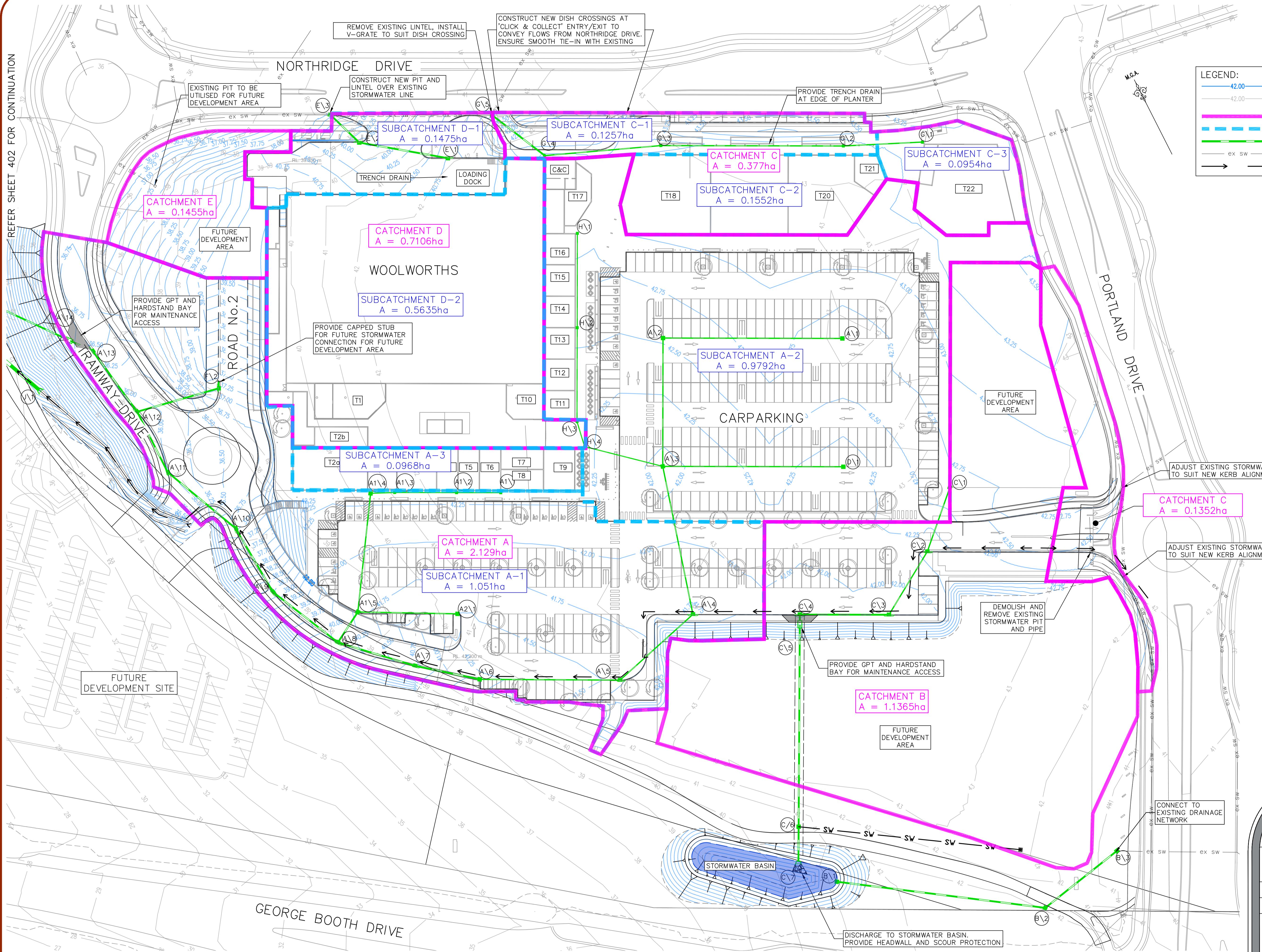
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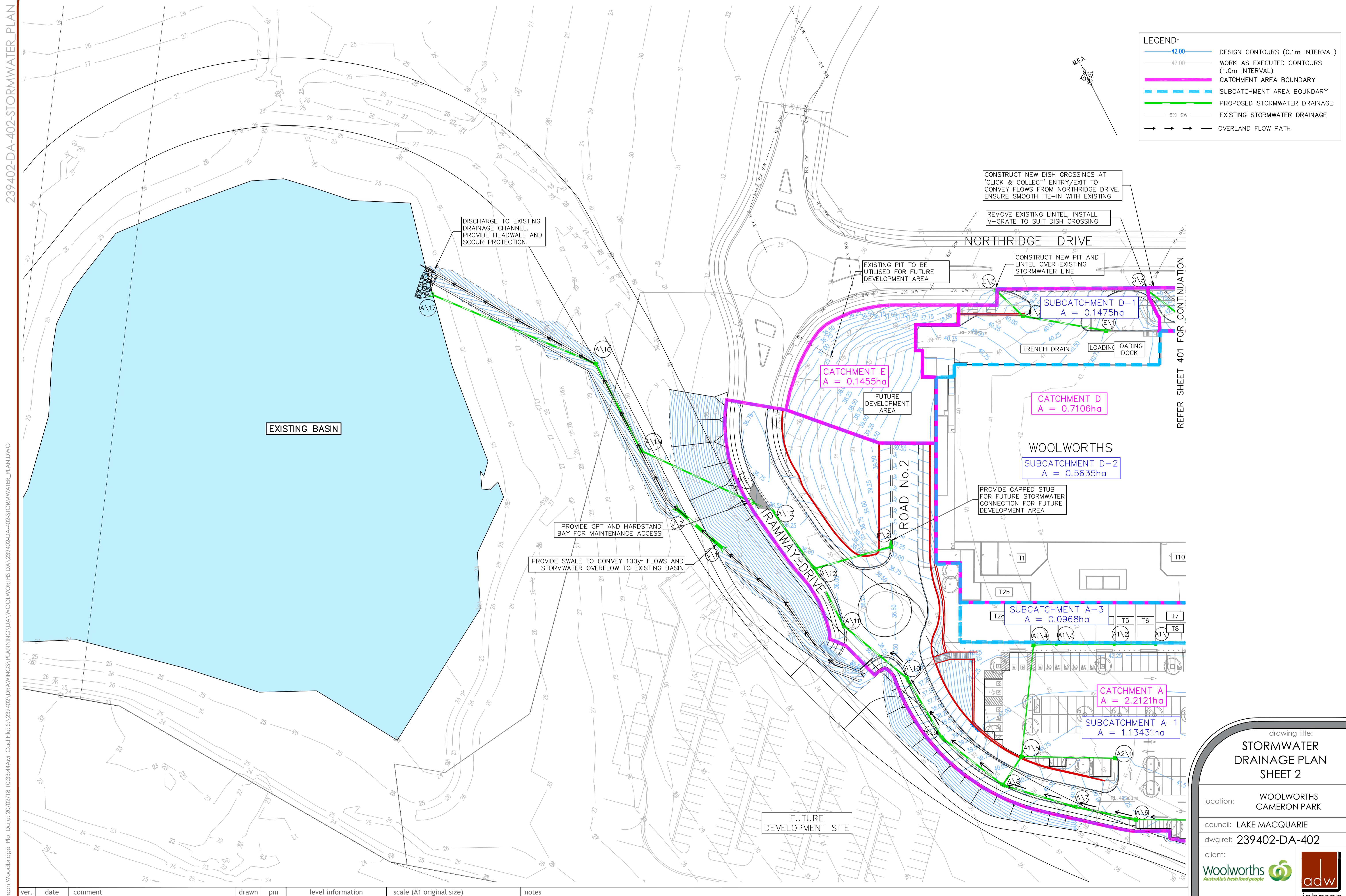
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		• town planning	• surveying	• development feasibility	• visualisation	• urban design	





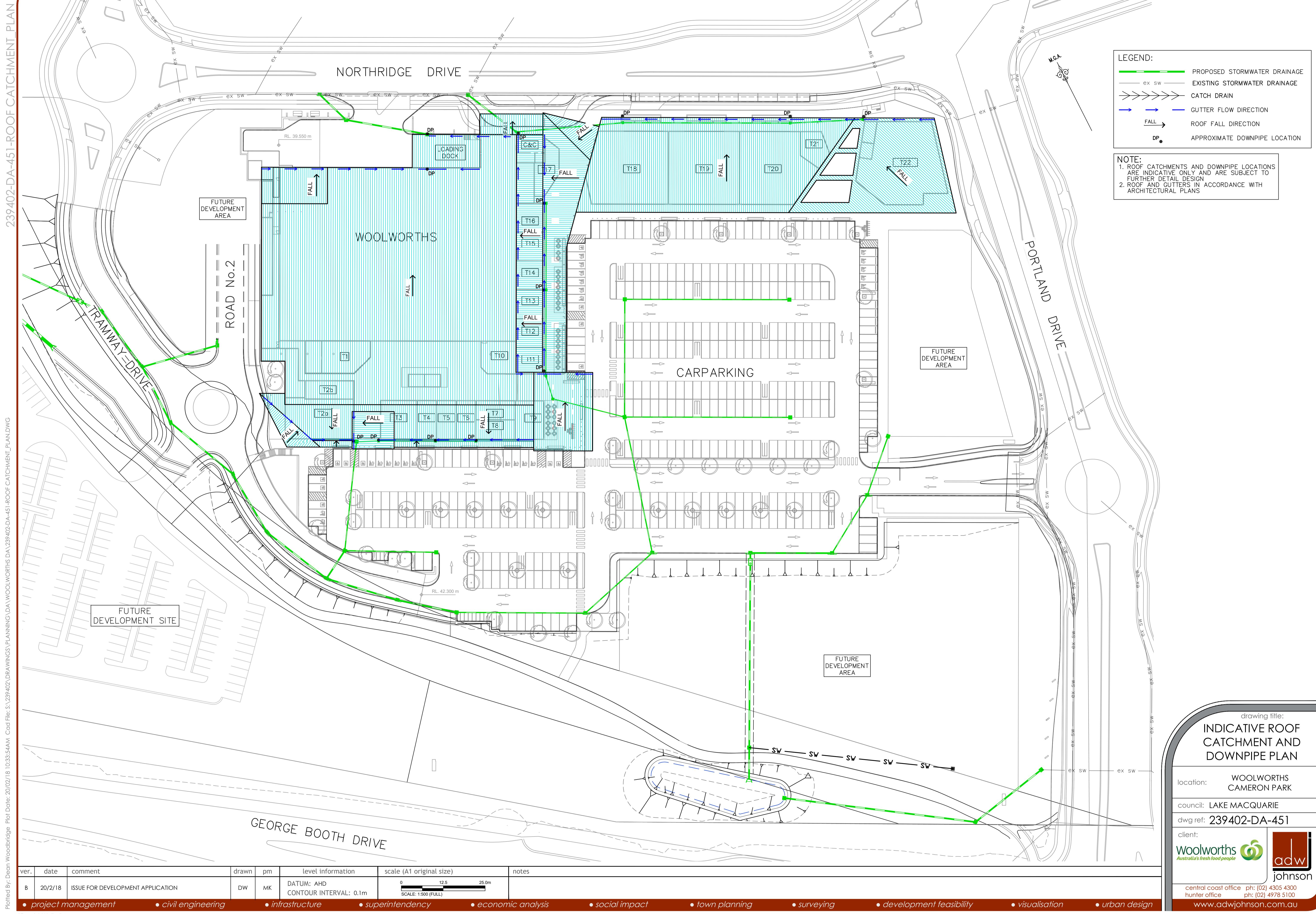
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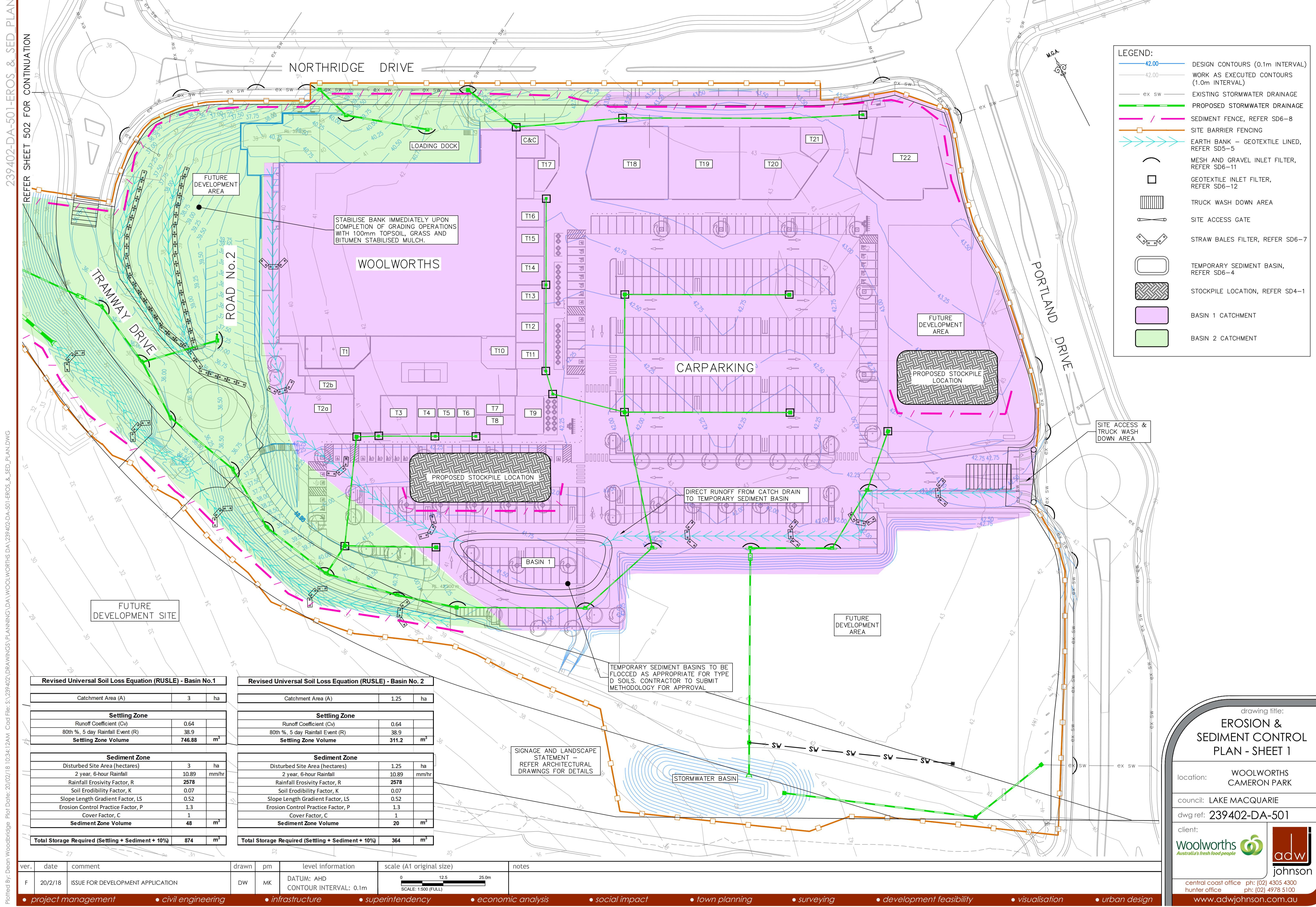
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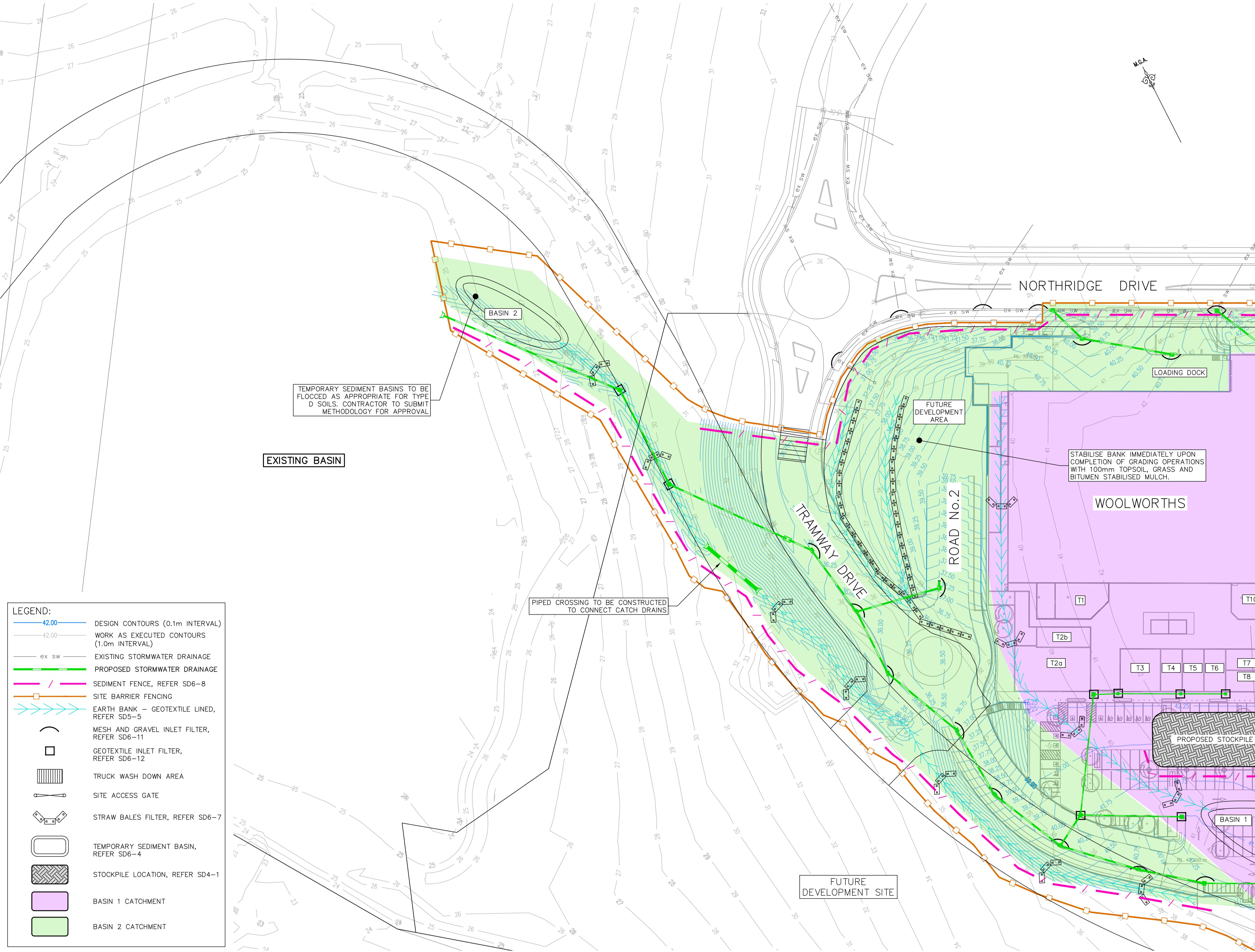
drawing title: STORMWATER DRAINAGE PLAN SHEET 2

location: WOOLWORTHS CAMERON PARK
council: LAKE MACQUARIE
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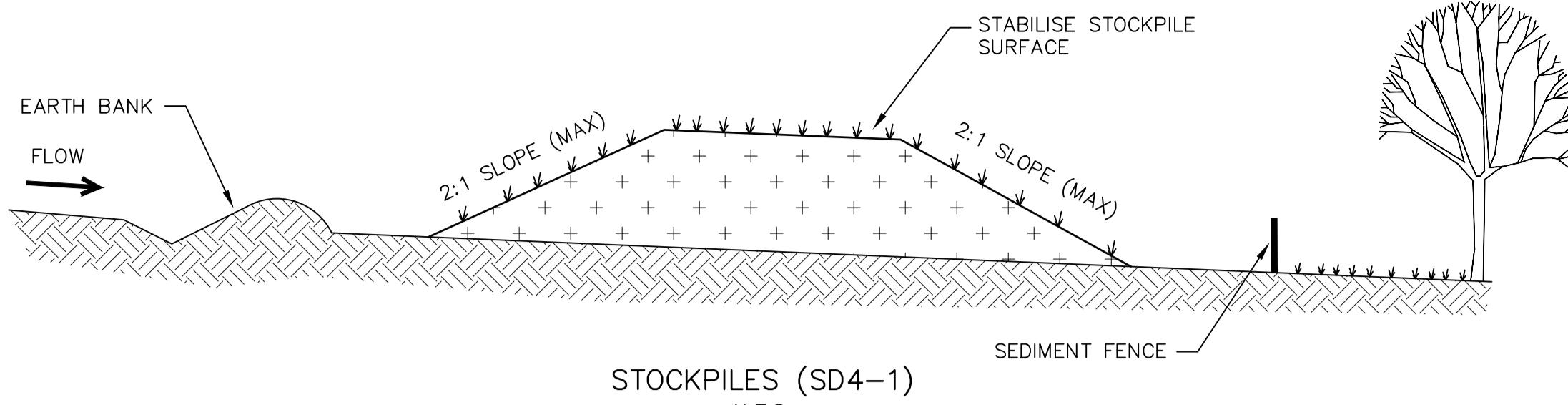




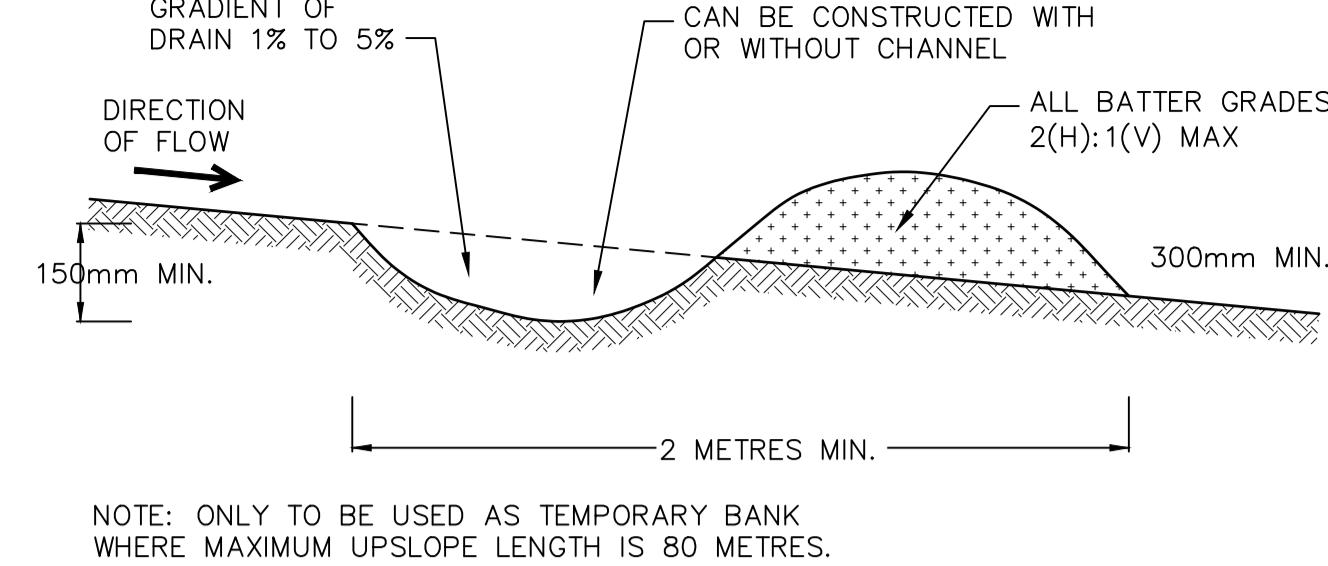
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REFER SHEET 501 FOR CONTINUATION



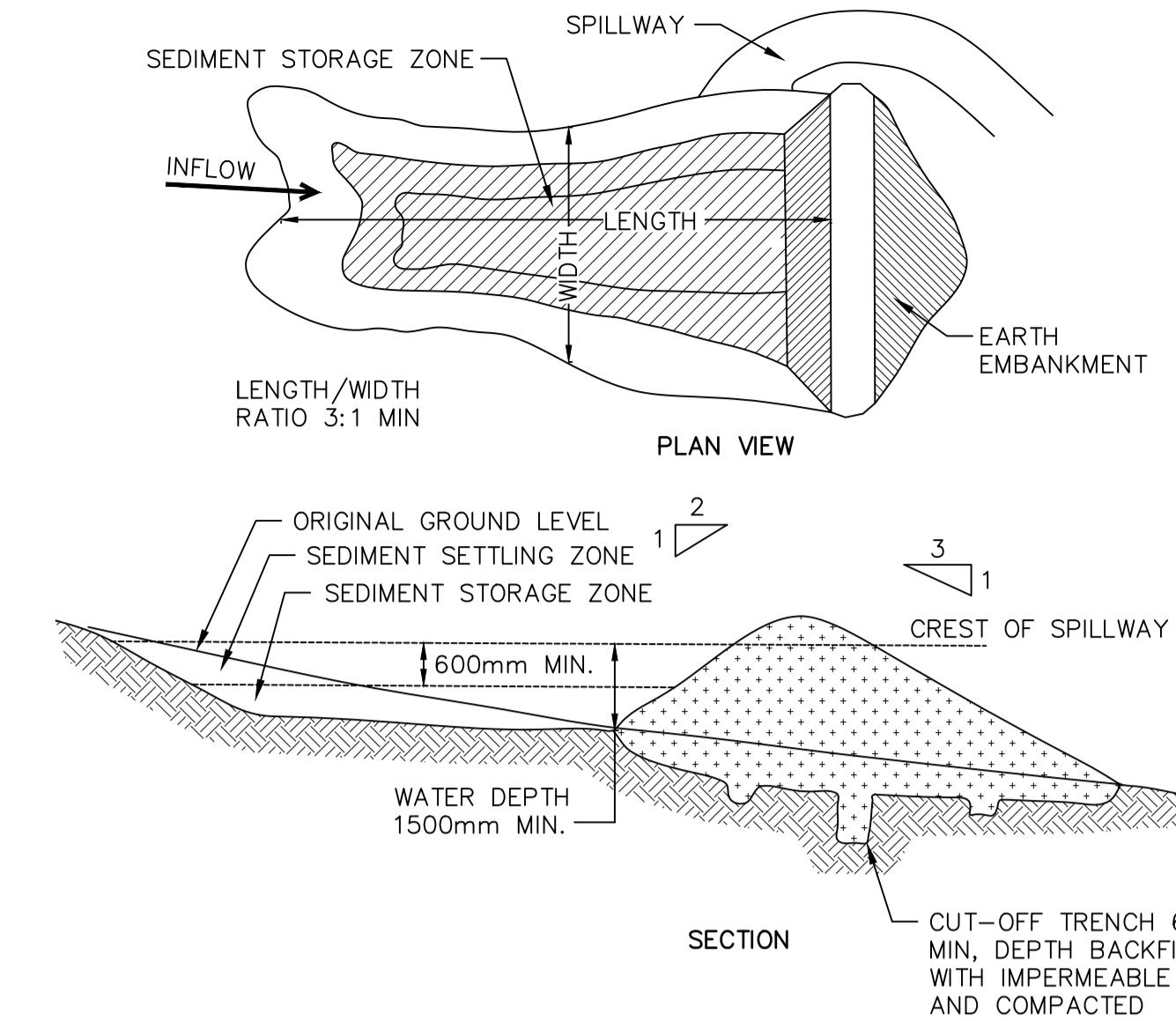


- CONSTRUCTION NOTES:**
1. PLACE STOCKPILES MORE THAN 2 (PREFERABLY 5) METRES FROM EXISTING VEGETATION, CONCENTRATED WATER FLOW, ROADS AND HAZARD AREAS.
 2. CONSTRUCT ON THE CONTOUR AS LOW, FLAT, ELONGATED MOUNDS.
 3. WHERE THERE IS SUFFICIENT AREA, TOPSOIL STOCKPILES SHALL BE LESS THAN 2 METRES IN HEIGHT.
 4. WHERE THEY ARE TO BE IN PLACE FOR MORE THAN 10 DAYS, STABILISE FOLLOWING THE APPROVED ESCP OR SWMP TO REDUCE THE C-FACTOR TO LESS THAN 0.10.
 5. CONSTRUCT EARTH BANKS (STANDARD DRAWING 5-5) ON THE UPSLOP SIDE TO DIVERT WATER AROUND STOCKPILES AND SEDIMENT FENCES (STANDARD DRAWING 6-8) 1 TO 2 METRES DOWNSLOPE.



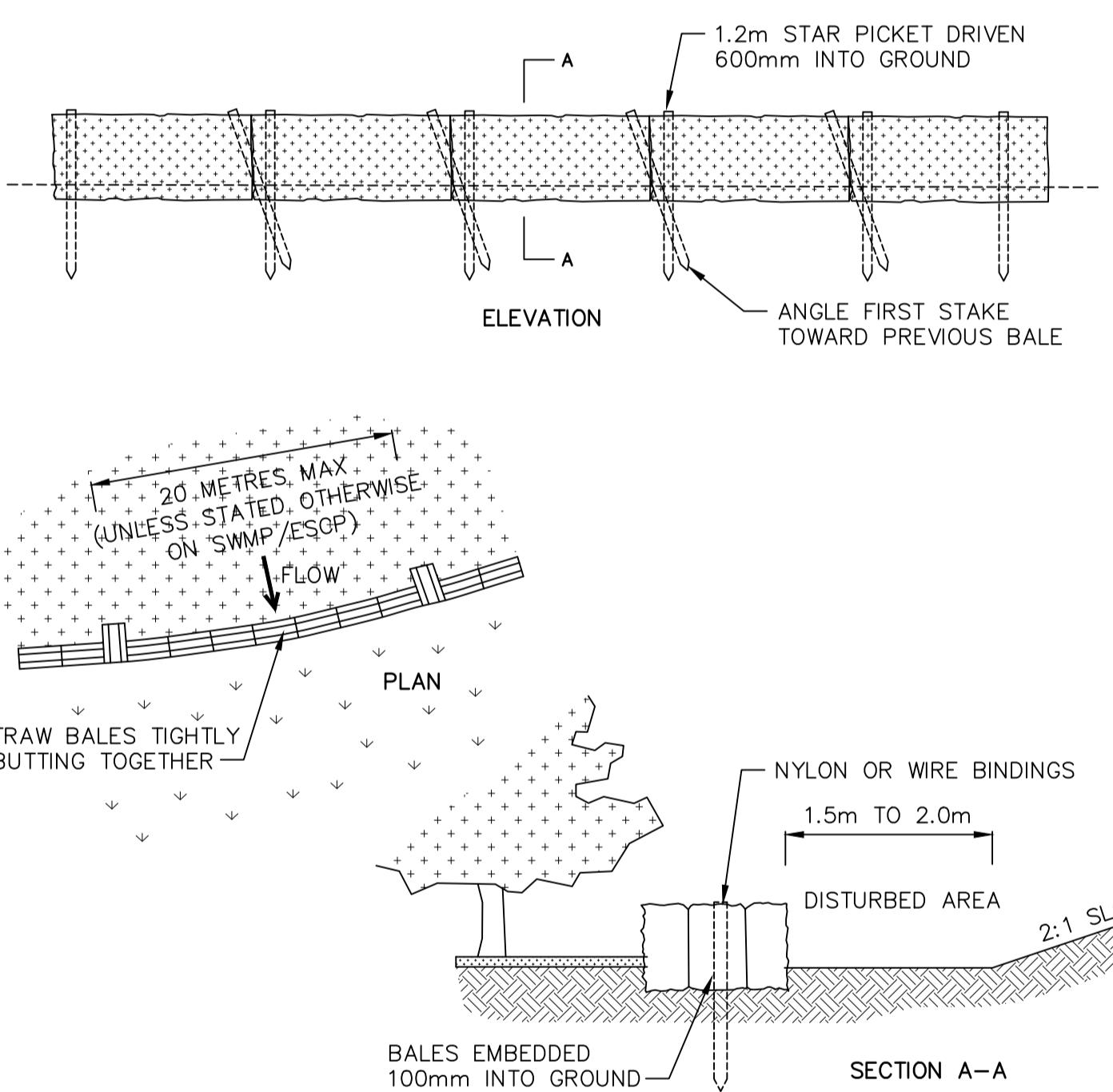
EARTH BANK (SD5-5)
N.T.S.

- CONSTRUCTION NOTES:**
1. BUILD WITH GRADIENTS BETWEEN 1 PERCENT AND 5 PERCENT.
 2. AVOID REMOVING TREES AND SHRUBS IF POSSIBLE – WORK AROUND THEM.
 3. ENSURE THE STRUCTURES ARE FREE OF PROJECTIONS OR OTHER IRREGULARITIES THAT COULD IMPEDE WATER FLOW.
 4. BUILD THE DRAINS WITH CIRCULAR, PARABOLIC OR TRAPEZOIDAL CROSS SECTIONS, NOT V SHAPED.
 5. ENSURE THE BANKS ARE PROPERLY COMPAKTED TO PREVENT FAILURE.
 6. COMPLETE PERMANENT OR TEMPORARY STABILISATION WITHIN 10 DAYS OF CONSTRUCTION.



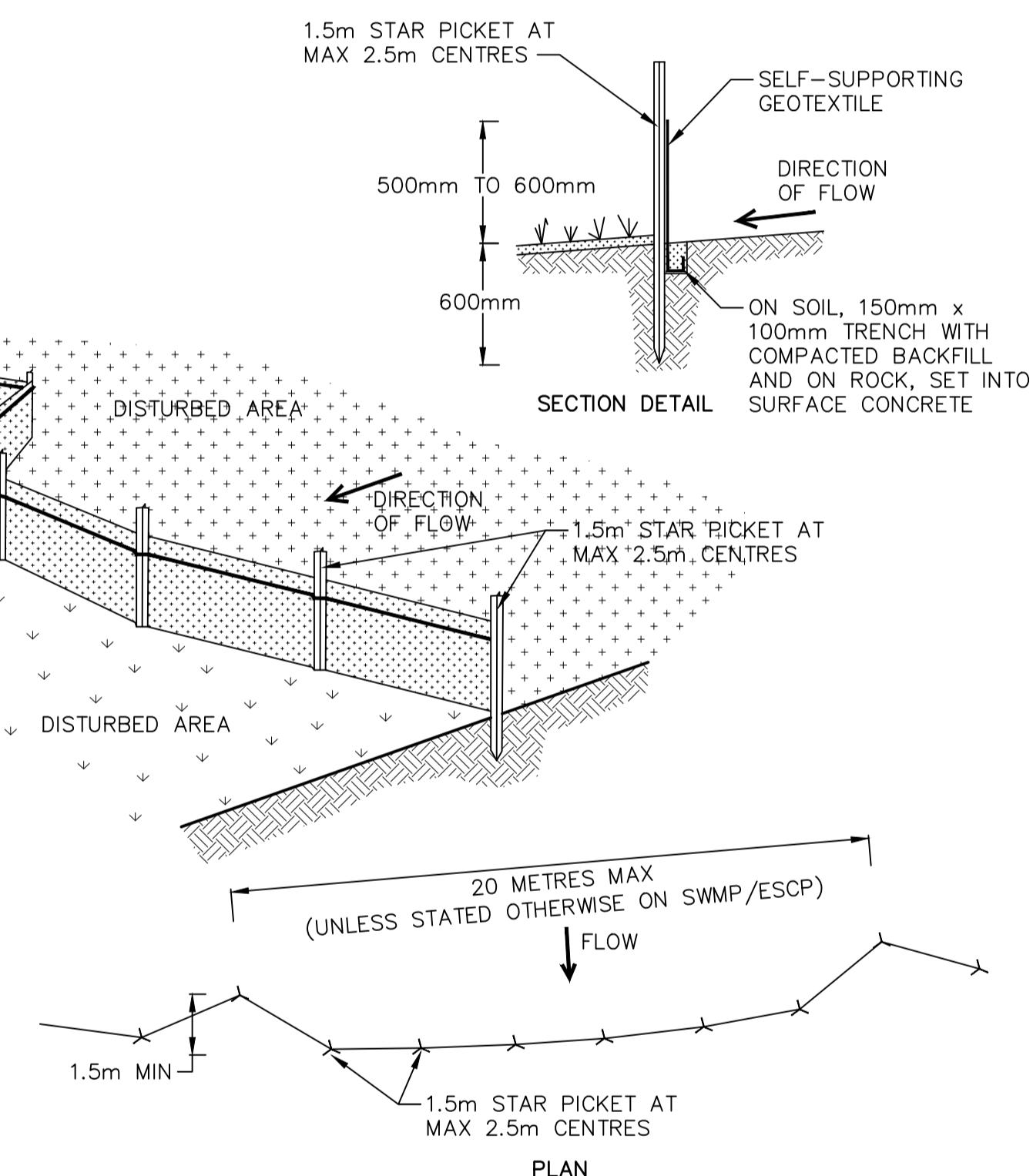
WET EARTH BASIN (SD6-4)
(APPLIES TO 'TYPE D' SOILS ONLY)
N.T.S.

- CONSTRUCTION NOTES:**
1. REMOVE ALL VEGETATION AND TOPSOIL FROM UNDER THE DAM WALL AND FROM WITHIN THE STORAGE AREA.
 2. CONSTRUCT A CUT-OFF TRENCH 500mm DEEP AND 1200mm WIDE ALONG THE CENTRELINE OF THE EMBANKMENT EXTENDING TO A POINT ON THE GULLY WALL LEVEL WITH THE RISER CREST.
 3. MAINTAIN THE TRENCH FREE OF WATER AND RECOMPACT THE MATERIALS WITH EQUIPMENT AS SPECIFIED IN THE SWMP TO 95 PERCENT STANDARD PROCTOR DENSITY.
 4. SELECT FILL ACCORDING TO THE SWMP THAT IS FREE FROM ROOTS, WOOD, ROCK, LARGE STONE OR FOREIGN MATERIAL.
 5. PREPARE THE SITE UNDER THE EMBANKMENT BY RIPPING TO AT LEAST 100mm TO HELP BOND THE COMPACTED FILL TO THE EXISTING SUBSTRATE.
 6. SPREAD THE FILL IN 100mm TO 150mm LAYERS AND COMPACT IT AT OPTIMUM MOISTURE CONTENT FOLLOWING THE SWMP.
 7. CONSTRUCT THE EMERGENCY SPILLWAY.
 8. REHABILITATE THE STRUCTURE FOLLOWING THE SWMP.



STRAW BALE FILTER (SD6-7)
N.T.S.

- CONSTRUCTION NOTES:**
1. CONSTRUCT THE STRAW BALE FILTER AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE.
 2. PLACE BALES LENGTHWISE IN A ROW WITH ENDS TIGHTLY ABUTTING, USE STRAW TO FILL ANY GAPS BETWEEN BALES. STRAWS ARE TO BE PLACED PARALLEL TO GROUND.
 3. ENSURE THAT THE MAXIMUM HEIGHT OF THE FILTER IS ONE BALE.
 4. EMBED EACH BALE IN THE GROUND 75mm TO 100mm AND ANCHOR WITH TWO 1.2m STAR PICKETS OR STAKES. ANGLE THE FIRST STAR PICKET OR STAKE IN EACH BALE TOWARDS THE PREVIOUSLY LAID BALE, DRIVE THEM 600mm IN THE GROUND AND, IF POSSIBLE, FLUSH WITH THE TOP OF THE BALES. WHERE STAR PICKETS ARE USED AND THEY PROTRUDE ABOVE THE BALES, ENSURE THEY ARE FITTED WITH SAFETY CAPS.
 5. WHERE A STRAW BALE FILTER IS CONSTRUCTED DOWNSLOPE FROM A DISTURBED BATTER, ENSURE THE BALES ARE PLACES 1 TO 2 METRES DOWNSLOPE FROM THE TOE.
 6. ESTABLISH A MAINTENANCE PROGRAM THAT ENSURES THE INTEGRITY OF THE BALES IS RETAINED – THEY COULD REQUIRE REPLACEMENT EACH TWO TO FOUR MONTHS.



SEDIMENT FENCE (SD6-8)
N.T.S.

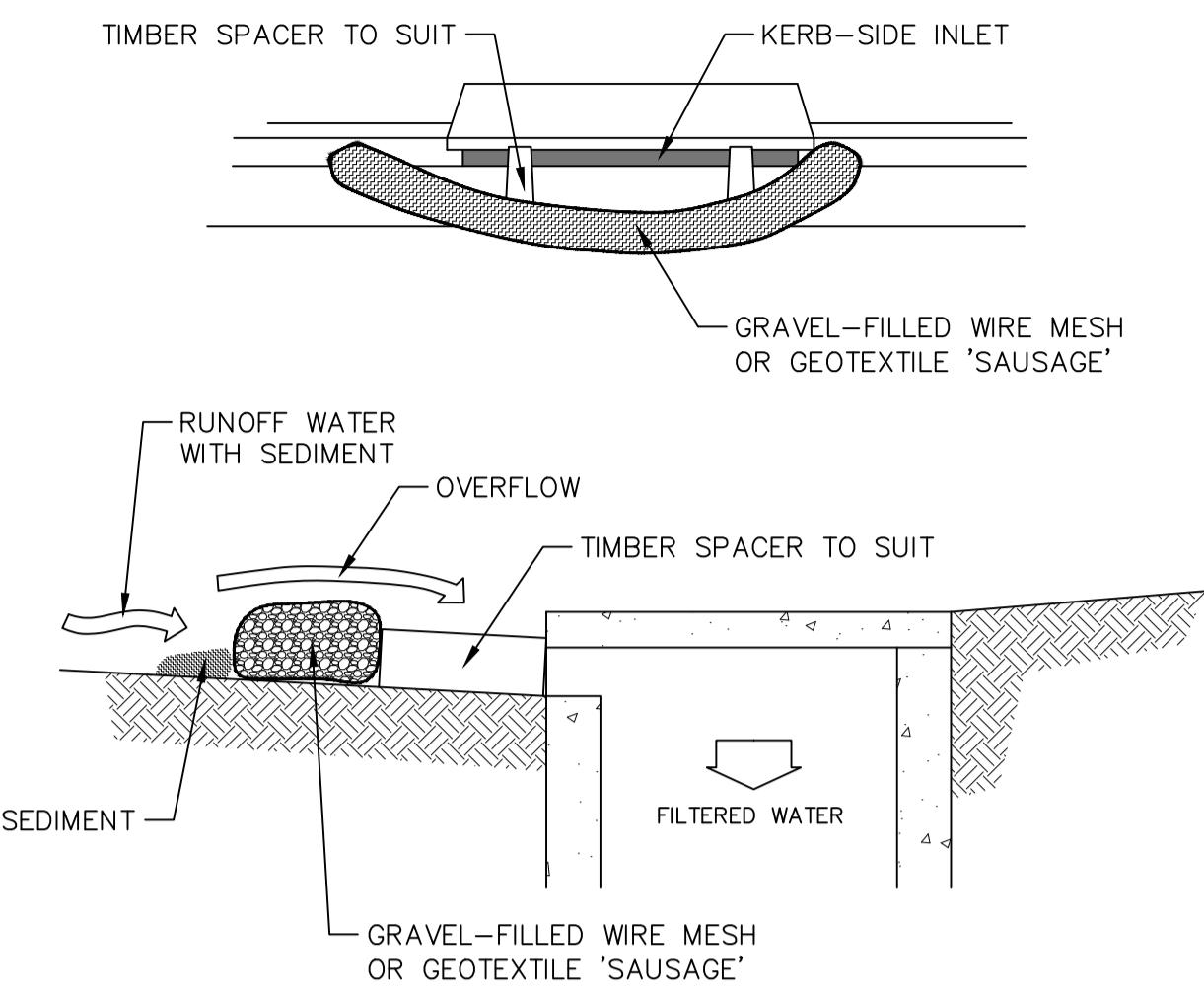
- CONSTRUCTION NOTES:**
1. CONSTRUCT SEDIMENT FENCES AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING TO LIMIT THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50 LITRES PER SECOND IN THE DESIGN STORM EVENT, USUALLY THE 10-YEAR EVENT.
 2. CUT A 150mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF THE FABRIC TO BE ENTRENCHED.
 3. DRIVE 1.5m LONG STAR PICKETS INTO GROUND AT 2.5 METRE INTERVALS (MAX) AT DOWNSLOPE EDGE OF THE TRENCH. ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY CAPS.
 4. FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
 5. JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.
 6. BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.

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drawing title:
**EROSION &
SEDIMENT CONTROL
DETAILS - SHEET 1**

location:	WOOLWORTHS CAMERON PARK
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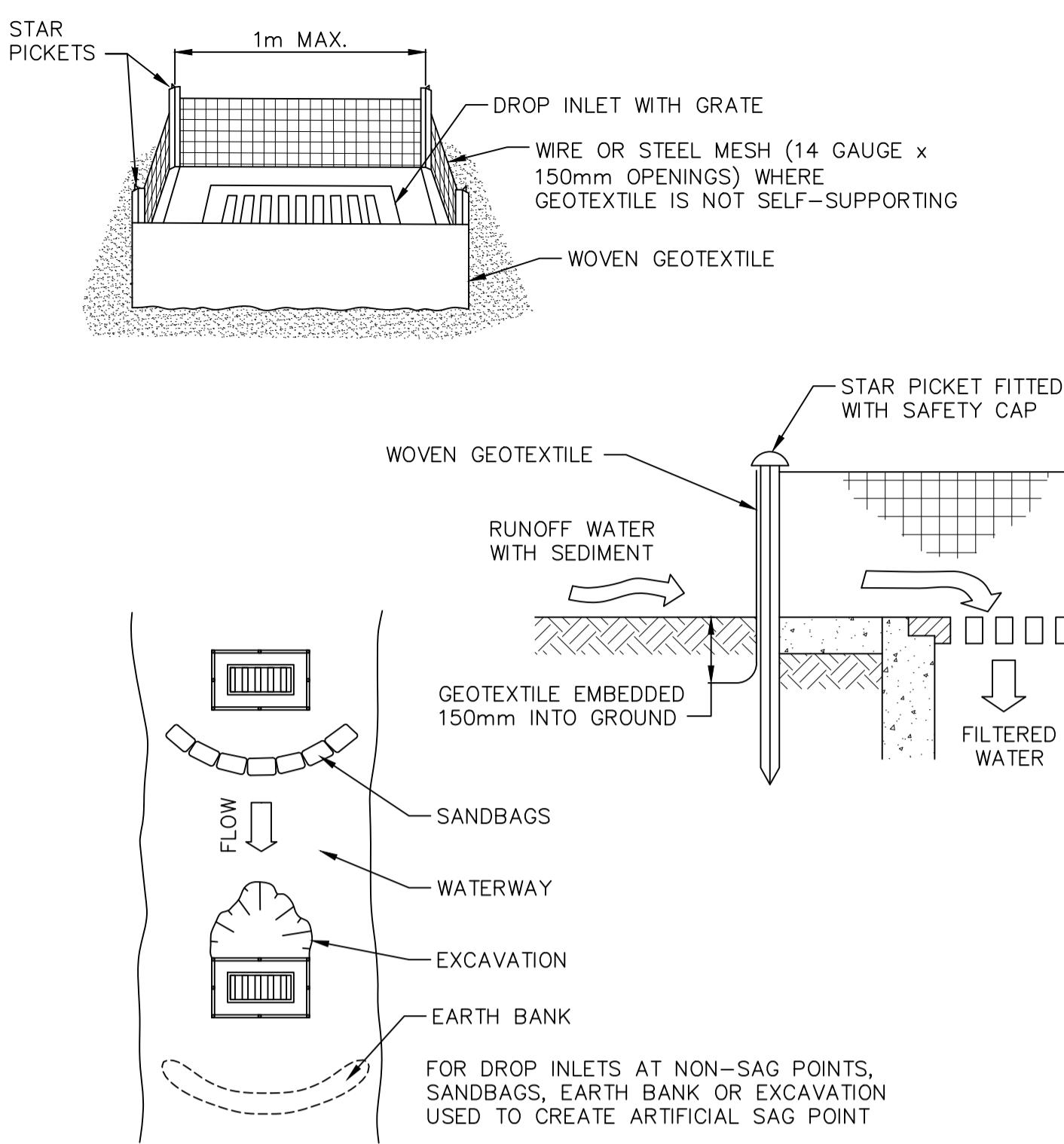


NOTE: THIS PRACTICE ONLY TO BE USED WHERE
SPECIFIED IN AN APPROVED SWMP/ESCP.

MESH AND GRAVEL INLET FILTER (SD6-11)
N.T.S.

CONSTRUCTION NOTES:

1. INSTALL FILTERS TO KERB INLETS ONLY ONLY AT SAG POINTS.
2. FABRICATE A SLEEVE MADE FROM GEOTEXTILE OR WIRE MESH LONGER THAN THE LENGTH OF THE INLET PIT AND FILL IT WITH 25mm TO 50mm GRAVEL.
3. FORM AN ELLIPTICAL CROSS SECTION ABOUT 150mm HIGH x 400mm WIDE.
4. PLACE THE FILTER AT THE OPENING LEAVING AT LEAST A 100mm SPACE BETWEEN IT AND THE KERB INLET. MAINTAIN THE OPENING WITH SPACER BLOCKS.
5. FORM A SEAL WITH THE KERB TO PREVENT SEDIMENT BYPASSING THE FILTER.
6. SANDBAGS FILLED WITH GRAVEL CAN SUBSTITUTE FOR THE MESH OR GEOTEXTILE PROVIDING THEY ARE PLACED SO THAT THEY FIRMLY ABUT EACH OTHER AND SEDIMENT-LADEN WATERS CANNOT PASS BETWEEN.



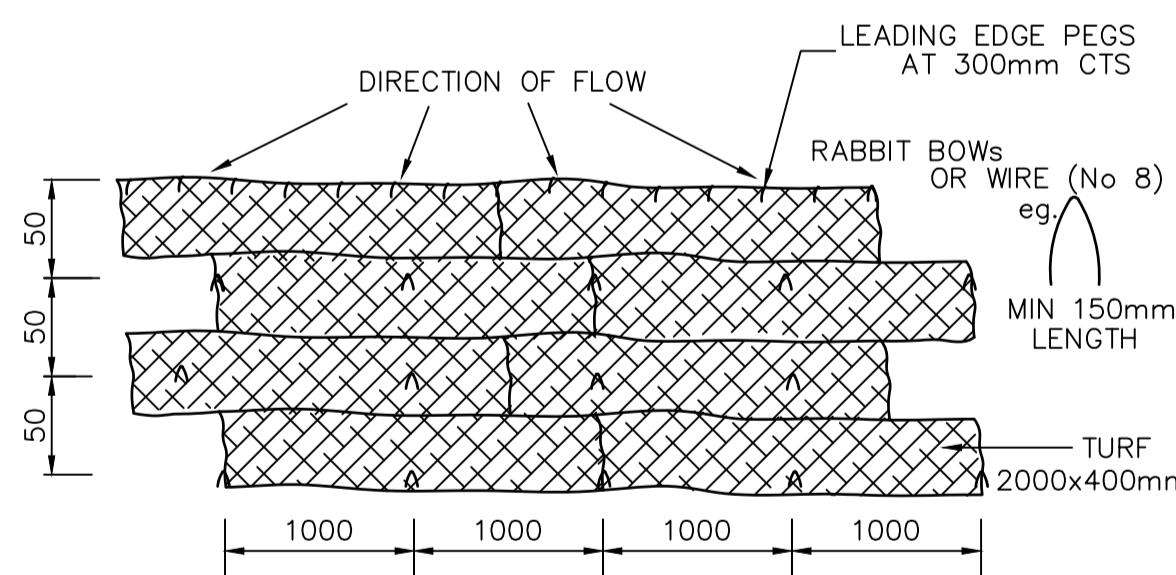
GEOTEXTILE INLET FILTER (SD6-12)

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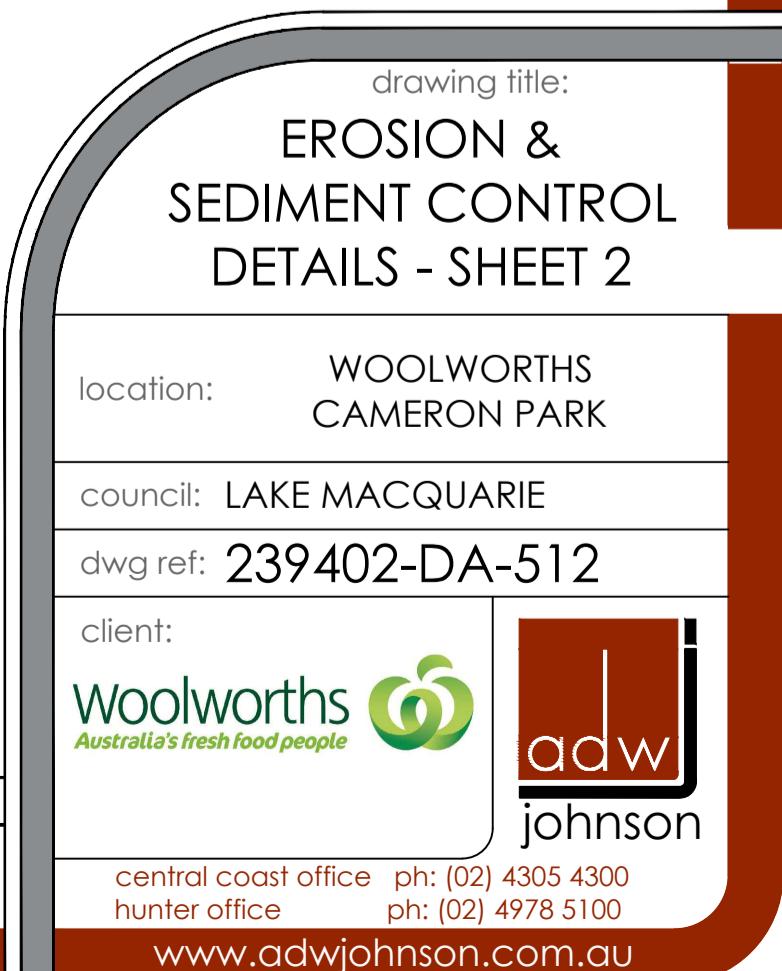
CONSTRUCTION NOTES:

- CONSTRUCTION NOTES:**

 1. FABRICATE A SEDIMENT BARRIER MADE FROM GEOTEXTILE OR STRAW BALES.
 2. REFER STANDARD DRAWINGS 6-7 & 6-8 FOR INSTALLATION PROCEDURES FOR THE STRAW BALES OR GEOFABRIC. REDUCE THE PICKET SPACING TO 1 METRE CENTRES.
 3. IN WATERWAYS, ARTIFICIAL SAG POINTS CAN BE CREATED WITH SANDBAGS OR EARTH BANKS AS SHOWN IN THE DRAWING.
 4. DO NOT COVER THE INLET WITH GEOTEXTILE UNLESS THE DESIGN IS ADEQUATE TO ALLOW FOR ALL WATERS TO BYPASS IT.



TURF LAYING CONFIGURATION



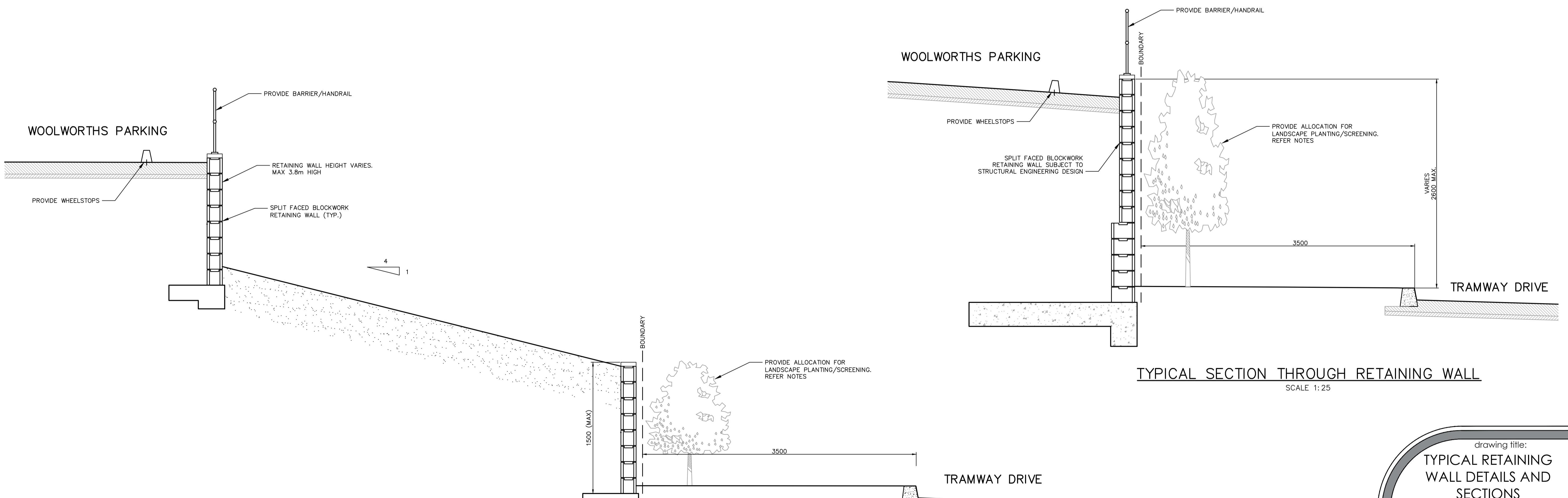
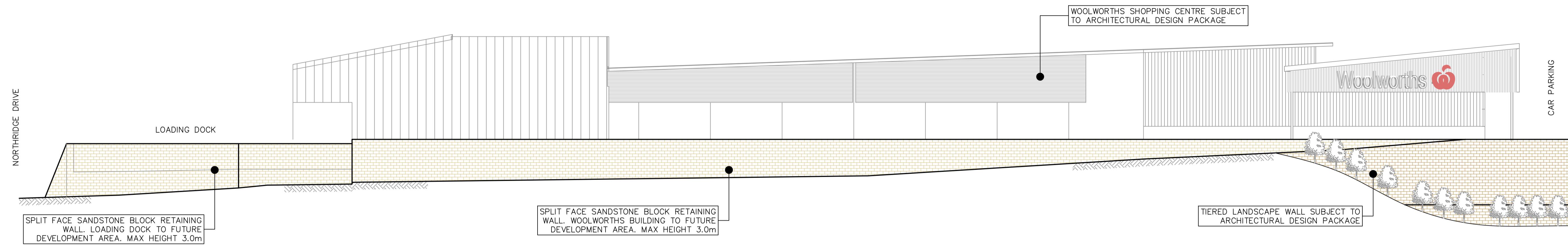
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GENERAL

1. ESCP REFERS TO EROSION AND SEDIMENT CONTROL PLAN OR A SOIL AND WATER MANAGEMENT PLAN (SWMP).
 2. ESC REFERS TO EROSION AND SEDIMENT CONTROL.
 3. SEDIMENT, INCLUDES, BUT IS NOT LIMITED TO, CLAY, SILT, SAND, GRAVEL, SOIL, MUD, CEMENT, AND CERAMIC WASTE.
 4. ANY REFERENCE TO THE BLUE BOOK REFERS TO MANAGING URBAN STORMWATER – SOILS AND CONSTRUCTION. LANDCOM, 2004.
 5. ANY REFERENCE TO THE IECA WHITE BOOKS (2008) REFERS TO IECA 2008. BEST PRACTICE EROSION AND SEDIMENT CONTROL. BOOKS 1-6. INTERNATIONAL EROSION CONTROL ASSOCIATION (AUSTRALASIA). PICTON NSW.
 6. ANY MATERIAL DEPOSITED IN ANY CONSERVATION AREA FROM WORKS ASSOCIATED WITH THE DEVELOPMENT SHALL BE REMOVED IMMEDIATELY BY MEASURES INVOLVING MINIMAL GROUND AND/OR VEGETATION DISTURBANCE AND NO MACHINERY, OR FOLLOWING DIRECTIONS BY COUNCIL AND/OR WITHIN A TIMEFRAME ADVISED BY COUNCIL.
 - THE ESCP**
 7. THE ESCP AND ITS ASSOCIATED ESC MEASURES SHALL BE CONSTANTLY MONITORED, REVIEWED, AND MODIFIED AS REQUIRED TO CORRECT DEFICIENCIES. COUNCIL HAS THE RIGHT TO DIRECT CHANGES IF, IN ITS OPINION, THE MEASURES THAT ARE PROPOSED OR HAVE BEEN INSTALLED ARE INADEQUATE TO PREVENT POLLUTION.
 8. PRIOR TO ANY ACTIVITIES ONSITE, THE RESPONSIBLE PERSON(S) IS TO BE NOMINATED. THE RESPONSIBLE PERSON(S) SHALL BE RESPONSIBLE FOR THE ESC MEASURES ONSITE. THE NAME, ADDRESS AND 24 HOUR CONTACT DETAILS OF THE PERSON(S) SHALL BE PROVIDED TO COUNCIL IN WRITING. COUNCIL SHALL BE ADVISED WITHIN 48 HOURS OF ANY CHANGES TO THE RESPONSIBLE PERSON(S), OR THEIR CONTACT DETAILS, IN WRITING.
 9. AT LEAST 14 DAYS BEFORE THE NATURAL SURFACE IS DISTURBED IN ANY NEW STAGE, THE CONTRACTOR SHALL SUBMIT TO THE CERTIFIER, A PLAN SHOWING ESC MEASURES FOR THAT STAGE. THE DEGREE OF DESIGN DETAIL SHALL BE BASED ON THE DISTURBED AREA.
 10. AT ANY TIME DURING CONSTRUCTION, THE ESC MEASURES ONSITE SHALL BE APPROPRIATE FOR THE AREA OF DISTURBANCE AND ITS CHARACTERISTICS INCLUDING SOILS (IN ACCORDANCE WITH THOSE REQUIRED FOR THE SITE AS PER DCP).
 11. THE IMPLEMENTATION OF THE ESCP SHALL BE SUPERVISED BY PERSONNEL WITH APPROPRIATE QUALIFICATIONS AND/OR EXPERIENCE IN ESC ON CONSTRUCTION SITES.
 12. THE APPROVED ESCP SHALL BE AVAILABLE ON-SITE FOR INSPECTION BY COUNCIL OFFICERS WHILE WORK ACTIVITIES ARE OCCURRING.
 13. THE APPROVED ESCP SHALL BE UP TO DATE AND SHOW A TIMELINE OF INSTALLATION, MAINTENANCE AND REMOVAL OF ESC MEASURES.
 14. ALL ESC MEASURES SHALL BE APPROPRIATE FOR THE SEDIMENT TYPE(S) OF THE SOILS ONSITE, IN ACCORDANCE WITH THE BLUE BOOK, IECA WHITE BOOKS OR OTHER CURRENT RECOGNISED INDUSTRY STANDARD FOR ESC FOR AUSTRALIAN CONDITIONS.
 15. ADEQUATE SITE DATA, INCLUDING SOIL DATA FROM A NATA APPROVED LABORATORY, SHALL BE OBTAINED TO ALLOW THE PREPARATION OF AN APPROPRIATE ESCP, AND ALLOW THE SELECTION, DESIGN AND SPECIFICATION OF REQUIRED ESC MEASURES.
 16. ALL WORKS SHALL BE CARRIED OUT IN ACCORDANCE WITH THE APPROVED ESCP (AS AMENDED FROM TIME TO TIME) UNLESS CIRCUMSTANCES ARISE WHERE:
 - a) COMPLIANCE WITH THE ESCP WOULD INCREASE THE POTENTIAL FOR ENVIRONMENTAL HARM; OR
 - b) CIRCUMSTANCES CHANGE DURING CONSTRUCTION AND THOSE CIRCUMSTANCES COULD NOT HAVE BEEN FORESEEN; OR
 - c) COUNCIL DETERMINES THAT UNACCEPTABLE OFF-SITE SEDIMENTATION IS OCCURRING AS A RESULT OF A LAND-DISTURBING ACTIVITY. IN EITHER CASE, THE PERSON(S) RESPONSIBLE MAY BE REQUIRED TO TAKE ADDITIONAL, OR ALTERNATIVE PROTECTIVE ACTION, AND/OR UNDERTAKE REASONABLE RESTORATION WORKS WITHIN THE TIMEFRAME SPECIFIED BY THE COUNCIL.
 17. ADDITIONAL ESC MEASURES SHALL BE IMPLEMENTED, AND A REVISED ESCP SUBMITTED FOR APPROVAL TO THE CERTIFIER (WITHIN FIVE BUSINESS DAYS OF ANY SUCH AMENDMENTS) IN THE EVENT THAT:
 - a) THERE IS A HIGH PROBABILITY THAT SERIOUS OR MATERIAL ENVIRONMENTAL HARM MAY OCCUR AS A RESULT OF SEDIMENT LEAVING THE SITE; OR
 - b) THE IMPLEMENTED WORKS FAIL TO ACHIEVE COUNCIL'S WATER QUALITY OBJECTIVES SPECIFIED IN THESE CONDITIONS; OR
 - c) SITE CONDITIONS SIGNIFICANTLY CHANGE; OR
 - d) SITE INSPECTIONS INDICATE THAT THE IMPLEMENTED WORKS ARE FAILING TO ACHIEVE THE "OBJECTIVE" OF THE ESCP.
 18. A COPY OF ANY AMENDED ESCP SHALL BE FORWARDED TO AN APPROPRIATE COUNCIL OFFICER, WITHIN FIVE BUSINESS DAYS OF ANY SUCH AMENDMENTS.
- SITE ESTABLISHMENT INCLUDING CLEARING AND MULCHING**
19. NO LAND CLEARING SHALL BE UNDERTAKEN UNLESS PRECEDED BY THE INSTALLATION OF ADEQUATE DRAINAGE AND SEDIMENT CONTROL MEASURES, UNLESS SUCH CLEARING IS REQUIRED FOR THE PURPOSE OF INSTALLING SUCH MEASURES, IN WHICH CASE, ONLY THE MINIMUM CLEARING REQUIRED TO INSTALL SUCH MEASURES SHALL OCCUR.
 20. BULK TREE CLEARING AND GRUBBING OF THE SITE SHALL BE IMMEDIATELY FOLLOWED BY SPECIFIED TEMPORARY EROSION CONTROL MEASURES (E.G. TEMPORARY GRASSING OR MULCHING) PRIOR TO COMMENCEMENT OF EACH STAGE OF CONSTRUCTION WORKS.
 21. TREES AND VEGETATION CLEARED FROM THE SITE SHALL BE MULCHED ONSITE WITHIN 7 DAYS OF CLEARING.
 22. APPROPRIATE MEASURES SHALL BE UNDERTAKEN TO CONTROL ANY DUST ORIGINATING DUE TO THE MULCHING OF VEGETATION ONSITE.
 23. ALL OFFICE FACILITIES AND OPERATIONAL ACTIVITIES SHALL BE LOCATED SUCH THAT ANY EFFLUENT, INCLUDING WASH-DOWN WATER, CAN BE TOTALLY CONTAINED AND TREATED WITHIN THE SITE.
 24. ALL REASONABLE AND PRACTICABLE MEASURES SHALL BE TAKEN TO ENSURE STORMWATER RUNOFF FROM ACCESS ROADS AND STABILISED ENTRY/EXIT SYSTEMS, DRAINS TO AN APPROPRIATE SEDIMENT CONTROL DEVICE.
 25. SITE EXIT POINTS SHALL BE APPROPRIATELY MANAGED TO MINIMISE THE RISK OF SEDIMENT BEING TRACKED ONTO SEALED, PUBLIC ROADWAYS.
 26. STORMWATER RUNOFF FROM ACCESS ROADS AND STABILISED ENTRY/EXIT POINTS SHALL DRAIN TO AN APPROPRIATE SEDIMENT CONTROL DEVICE.
 27. THE APPLICANT SHALL ENSURE AN ADEQUATE SUPPLY OF ESC AND APPROPRIATE POLLUTION CLEAN-UP MATERIALS ARE AVAILABLE ON-SITE AT ALL TIMES.
 28. ALL TEMPORARY EARTH BANKS, FLOW DIVERSION SYSTEMS, AND SEDIMENT BASIN EMBANKMENTS SHALL BE MACHINE-COMPACTED, SEEDED AND MULCHED WITHIN TEN (10) DAYS OF FORMATION FOR THE PURPOSE OF ESTABLISHING A VEGETATIVE COVER, OR LINED APPROPRIATELY.
 29. SEDIMENT DEPOSITED OFF SITE AS A RESULT OF ON-SITE ACTIVITIES SHALL BE COLLECTED AND THE AREA CLEANED/REHABILITATED AS SOON AS REASONABLE AND PRACTICABLE.
 30. CONCRETE WASTE AND CHEMICAL PRODUCTS, INCLUDING PETROLEUM AND OIL-BASED PRODUCTS, SHALL BE PREVENTED FROM ENTERING ANY INTERNAL OR EXTERNAL WATER BODY, OR ANY EXTERNAL DRAINAGE SYSTEM, EXCLUDING THOSE ON-SITE WATER BODIES SPECIFICALLY DESIGNED TO CONTAIN AND/OR TREAT SUCH MATERIAL. APPROPRIATE MEASURES SHALL BE INSTALLED TO TRAP THESE MATERIALS ONSITE.
 31. BRICK, TILE OR MASONRY CUTTING SHALL BE CARRIED OUT ON A PVIOUS SURFACE (E.G. GRASS OR OPEN SOIL) AND IN SUCH A MANNER THAT ANY RESULTING SEDIMENT-LADEN RUNOFF IS PREVENTED FROM DISCHARGING INTO A GUTTER, DRAIN OR WATER. APPROPRIATE MEASURES SHALL BE INSTALLED TO TRAP THESE MATERIALS ONSITE.
 32. NEWLY SEALED HARD-STAND AREAS (E.G. ROADS, DRIVEWAYS AND CAR PARKS) SHALL BE SWEEPED THOROUGHLY AS SOON AS PRACTICABLE AFTER SEALING/SURFACING TO MINIMISE THE RISK OF COMPONENTS OF THE SURFACING COMPOUND ENTERING STORMWATER DRAINS.
 33. STOCKPILES OF ERODIBLE MATERIAL SHALL BE PROVIDED WITH AN APPROPRIATE PROTECTIVE COVER (SYNTHETIC OR ORGANIC) IF THE MATERIALS ARE LIKELY TO BE STOCKPILED FOR MORE THAN 10 DAYS.
 34. STOCKPILES, TEMPORARY OR PERMANENT, SHALL NOT BE LOCATED IN AREAS IDENTIFIED AS NO-GO ZONES (INCLUDING, BUT NOT LIMITED TO, RESTRICTED ACCESS AREAS, BUFFER ZONES, OR AREAS OF NON-DISTURBANCE) ON THE ESCP.
 35. NO MORE THAN 150M OF A STORMWATER, SEWER LINE OR OTHER SERVICE TRENCH SHALL BE OPEN AT ANY ONE TIME.
 36. SITE SPOIL SHALL BE LAWFULLY DISPOSED OF IN A MANNER THAT DOES NOT RESULT IN ONGOING SOIL EROSION OR ENVIRONMENTAL HARM.
 37. WHEREVER REASONABLE AND PRACTICABLE, STORMWATER RUNOFF ENTERING THE SITE FROM EXTERNAL AREAS, AND NON-SEDIMENT LADEN (CLEAN) STORMWATER RUNOFF ENTERING A WORK AREA OR AREA OF SOIL DISTURBANCE, SHALL BE DIVERTED AROUND OR THROUGH THAT AREA IN A MANNER THAT MINIMISES SOIL EROSION AND THE CONTAMINATION OF THAT WATER FOR ALL DISCHARGES UP TO THE SPECIFIED DESIGN STORM DISCHARGE.
- SITE MANAGEMENT INCLUDING DUST**
38. PRIORITY SHALL BE GIVEN TO THE PREVENTION, OR AT LEAST THE MINIMISATION, OF SOIL EROSION, RATHER THAN THE TRAPPING OF DISPLACED SEDIMENT. SUCH A CLAUSE SHALL NOT REDUCE THE RESPONSIBILITY TO APPLY AND MAINTAIN, AT ALL TIMES, ALL NECESSARY ESC MEASURES.
 39. MEASURES USED TO CONTROL WIND EROSION SHALL BE APPROPRIATE FOR THE LOCATION AND PREVENT SOIL EROSION AT ALL TIMES, INCLUDING WORKING HOURS, OUT OF HOURS, WEEKENDS, PUBLIC HOLIDAYS, AND DURING ANY OTHER SHUTDOWN PERIODS.
 40. THE APPLICATION OF LIQUID OR CHEMICAL-BASED DUST SUPPRESSION MEASURES SHALL ENSURE THAT SEDIMENT-LADEN RUNOFF RESULTING FROM SUCH MEASURES DOES NOT CREATE A TRAFFIC OR ENVIRONMENTAL HAZARD.
 41. ALL CUT AND FILL EARTH BATTERS LESS THAN 3M IN ELEVATION SHALL BE TOPSOILED, AND GRASS SEEDED/HYDROMULCHED WITHIN 10 DAYS OF COMPLETION OF GRADING IN CONSULTATION WITH COUNCIL.
 42. ONCE CUT/FILL OPERATIONS HAVE BEEN FINALISED IN A SECTION, ALL DISTURBED AREAS THAT ARE NOT BEING WORKED ON SHALL BE STABILISED IN ACCORDANCE WITH TIME LINES IN THE BLUE BOOK.
 43. ALL REASONABLE AND PRACTICABLE MEASURES SHALL BE TAKEN TO PREVENT, OR AT LEAST MINIMISE, THE RELEASE OF SEDIMENT FROM THE SITE.
 44. SUITABLE ALL-WEATHER MAINTENANCE ACCESS SHALL BE PROVIDED TO ALL SEDIMENT CONTROL DEVICES.
 45. SEDIMENT CONTROL DEVICES, OTHER THAN SEDIMENT BASINS, SHALL BE DE-SIELTED AND MADE FULLY OPERATIONAL AS SOON AS REASONABLE AND PRACTICABLE AFTER A SEDIMENT-PRODUCING EVENT, WHETHER NATURAL OR ARTIFICIAL, IF THE DEVICE'S SEDIMENT RETENTION CAPACITY FALLS BELOW 75% OF ITS DESIGN RETENTION CAPACITY.
 46. ALL EROSION AND SEDIMENT CONTROL MEASURES, INCLUDING DRAINAGE CONTROL MEASURES, SHALL BE MAINTAINED IN PROPER WORKING ORDER AT ALL TIMES DURING THEIR OPERATIONAL LIVES.
 47. WASHING/FLUSHING OF SEALED ROADWAYS SHALL ONLY OCCUR WHERE SWEEPING HAS FAILED TO REMOVE SUFFICIENT SEDIMENT AND THERE IS A COMPELLING NEED TO REMOVE THE REMAINING SEDIMENT (E.G. FOR SAFETY REASONS). IN SUCH CIRCUMSTANCES, ALL REASONABLE AND PRACTICABLE SEDIMENT CONTROL MEASURES SHALL BE USED TO PREVENT, OR AT LEAST MINIMISE, THE RELEASE OF SEDIMENT INTO RECEIVING WATERS. ONLY THOSE MEASURES THAT WILL NOT CAUSE SAFETY AND PROPERTY FLOODING ISSUES SHALL BE EMPLOYED. SEDIMENT REMOVED FROM ROADWAYS SHALL BE DISPOSED OF IN A LAWFUL MANNER THAT DOES NOT CAUSE ONGOING SOIL EROSION OR ENVIRONMENTAL HARM.
 48. SEDIMENT REMOVED FROM SEDIMENT TRAPS AND PLACES OF SEDIMENT DEPOSITION SHALL BE DISPOSED OF IN A LAWFUL MANNER THAT DOES NOT CAUSE ONGOING SOIL EROSION OR ENVIRONMENTAL HARM.

SEDIMENT BASINS – INSTALLATION, MAINTENANCE AND REMOVAL INCLUDING SEDIMENT TRAPS

49. AS-CONSTRUCTED PLANS SHALL BE PREPARED FOR ALL CONSTRUCTED SEDIMENT BASINS AND ASSOCIATED EMERGENCY SPILLWAYS. SUCH PLANS SHALL VERIFY THE BASIN'S DIMENSIONS, LEVELS AND VOLUMES COMPLY WITH THE APPROVED DESIGN DRAWINGS. THESE PLANS MAY BE REQUESTED BY THE CERTIFIER OR COUNCIL.
 50. SEDIMENT BASINS SHALL BE CONSTRUCTED AND FULLY OPERATIONAL PRIOR TO ANY OTHER SOIL DISTURBANCE IN THEIR CATCHMENT.
 51. INSTALL AN INTERNAL GATED VALVE, OR SIMILAR, IN ANY OUTLET PIPE ONCE PIPES INSTALLED, OR INSTALL A SACRIFICIAL PIPE FROM BASIN THROUGH WALL TO EXTERNAL OUTLET POINT. THE VALVE SHALL BE CONNECTED TO A RISER MADE FROM SLOTTED PIPE IN THE BASIN. THE VALVE MAY BE OPENED ONCE CAPTURED WATER MEETS WATER QUALITY REQUIREMENTS. THE FINAL SETUP FOR TEMPORARY INTERNAL OUTLET STRUCTURES TO BE CONFIRMED PRIOR TO CONSTRUCTION WITH COUNCIL. THIS SETUP WILL ENABLE DISCHARGE OF TREATED WATER FROM SITE WITHOUT NEED FOR PUMPING.
 52. A SEDIMENT STORAGE LEVEL MARKER POST SHALL BE WITH A CROSS MEMBER SET JUST BELOW THE TOP OF THE SEDIMENT STORAGE ZONE (AS SPECIFIED ON THE APPROVED ESCP). AT LEAST A 75MM WIDE POST SHALL BE FIRMLY SET INTO THE BASIN FLOOR.
 53. THE SITE MANAGER SHALL OBTAIN THE RELEVANT APPROVALS FROM THE RELEVANT ORGANISATIONS TO DISCHARGE TREATED WATER FROM ANY EXISTING BASINS. ORGANISATIONS MAY INCLUDE, BUT NOT BE LIMITED TO, HUNTER WATER, AND COUNCIL.
 54. WHERE MORE THAN ONE STAGE IS TO BE DEVELOPED AT ONE TIME, OR BEFORE THE PRECEDING STAGE IS COMPLETE, THE SEDIMENT BASIN(S) FOR THESE STAGES SHALL HAVE SUFFICIENT CAPACITY TO CATER FOR ALL AREA DIRECTED TO THE BASIN(S).
 55. PRIOR TO ANY FORECAST WEATHER EVENT LIKELY TO RESULT IN RUNOFF, ANY BASINS/TRAPS SHALL BE DEWATERED TO PROVIDE SUFFICIENT CAPACITY TO CAPTURE SEDIMENT-LADEN WATER FROM THE SITE.
 56. SUFFICIENT QUANTITIES OF CHEMICALS/AGENTS TO TREAT CAPTURED WATER SHALL BE PLACED SUCH THAT WATER ENTERING THE BASIN MIXES WITH THE CHEMICAL/AGENTS AND IS CARRIED INTO THE BASIN TO SPEED UP CLARIFICATION.
 57. ANY BASIN SHALL BE DEWATERED WITHIN THE X-DAY RAINFALL DEPTH USED TO CALCULATE THE CAPACITY OF THE BASIN, AFTER A RAINFALL EVENT.
 58. SUFFICIENT QUANTITIES OF CHEMICALS/AGENTS TO TREAT TURBID WATER SHALL BE SECURELY STORED ON-SITE TO PROVIDE FOR AT LEAST THREE COMPLETE TREATMENTS OF ALL BASINS REQUIRING CHEMICALLY TREATMENT ONSITE.
 59. PRIOR TO THE CONTROLLED DISCHARGE (E.G. DE-WATERING ACTIVITIES) FROM EXCAVATIONS AND/OR SEDIMENT BASINS, THE FOLLOWING WATER QUALITY OBJECTIVES SHALL BE ACHIEVED:
 - a) TOTAL SUSPENDED SOLIDS (TSS) TO A MAXIMUM 50mg/L;
 - b) WATER PH BETWEEN 6.5 AND 8.5, UNLESS OTHERWISE REQUIRED BY THE COUNCIL;
 - c) TURBIDITY (MEASURED IN NTU) TO A MAXIMUM OF 60 NTU; AND
 - d) EC LEVELS NO GREATER THAN BACKGROUND LEVELS.
 60. THE DEVELOPMENT APPROVAL MAY REQUIRE TESTING OF ADDITIONAL WATER QUALITY ELEMENTS PRIOR TO DISCHARGE. E.G. HEAVY METALS.
 61. A SAMPLE OF THE RELEASED TREATED WATER SHALL BE KEPT ONSITE IN A CLEAR CONTAINER WITH THE SAMPLE DATE RECORDED ON IT.
 62. WATER QUALITY SAMPLES SHALL BE TAKEN AT A DEPTH NO LESS THAN 200MM BELOW THE WATER SURFACE OF THE BASIN.
 63. NO ALUMINIUM BASED PRODUCTS MAY BE USED TREAT CAPTURED WATER ONSITE WITHOUT THE PRIOR WRITTEN PERMISSION FROM AN APPROPRIATE COUNCIL OFFICER. THE APPLICANT SHALL HAVE A DEMONSTRATED ABILITY TO USE SUCH PRODUCTS CORRECTLY AND WITHOUT ENVIRONMENTAL HARM PRIOR TO ANY APPROVAL.
 64. THE CHEMICAL/AGENT USED IN TYPE D AND TYPE F BASINS TO TREAT CAPTURED WATER CAPTURED IN THE BASIN SHALL BE APPLIED IN CONCENTRATIONS SUFFICIENT TO ACHIEVE COUNCIL'S WATER QUALITY OBJECTIVES WITHIN THE X-DAY RAINFALL DEPTH USED TO CALCULATE THE CAPACITY OF THE BASIN, AFTER A RAINFALL EVENT.
 65. ALL MANUFACTURERS' INSTRUCTIONS SHALL BE FOLLOWED FOR ANY CHEMICALS/AGENTS USED ONSITE, EXCEPT WHERE APPROVED BY THE RESPONSIBLE PERSON OR AN APPROPRIATE COUNCIL OFFICER.
 66. THE APPLICANT SHALL ENSURE THAT ON EACH OCCASION A TYPE F OR TYPE D BASIN WAS NOT DE-WATERED PRIOR TO BEING SURCHARGED BY A FOLLOWING RAINFALL EVENT, A REPORT PRESENTED TO AN APPROPRIATE COUNCIL OFFICER WITHIN 5 DAYS IDENTIFYING THE CIRCUMSTANCES AND PROPOSED AMENDMENTS, IF ANY, TO THE BASIN'S OPERATING PROCEDURES.
 67. SETTLED SEDIMENT SHALL BE REMOVED AS SOON AS REASONABLE AND PRACTICABLE FROM ANY SEDIMENT BASIN IF:
 - a) IT IS ANTICIPATED THAT THE NEXT STORM EVENT IS LIKELY TO CAUSE SEDIMENT TO SETTLE ABOVE THE BASIN'S SEDIMENT STORAGE ZONE; OR
 - b) THE ELEVATION OF SETTLED SEDIMENT IS ABOVE THE TOP OF THE BASIN'S SEDIMENT STORAGE ZONE; OR
 - c) THE ELEVATION OF SETTLED SEDIMENT IS ABOVE THE BASIN'S SEDIMENT MARKER LINE.
 68. SCOUR PROTECTION MEASURES PLACED ON SEDIMENT BASIN EMERGENCY SPILLWAYS SHALL APPROPRIATELY PROTECT THE SPILLWAY CHUTE AND ITS SIDE BATTERS FROM SCOUR, AND SHALL EXTEND A MINIMUM OF 3M BEYOND THE DOWNSTREAM TOE OF THE BASIN'S EMBANKMENT.
 69. SUITABLE ALL-WEATHER MAINTENANCE ACCESS SHALL BE PROVIDED TO ALL SEDIMENT CONTROL DEVICES.
 70. MATERIALS, WHETHER LIQUID OR SOLID, REMOVED FROM ANY ESC MEASURES DURING MAINTENANCE OR DECOMMISSIONING, SHALL BE DISPOSED OF IN A MANNER THAT DOES NOT CAUSE ONGOING SOIL EROSION OR ENVIRONMENTAL HARM.
 71. ALL SEDIMENT BASINS SHALL REMAIN FULLY OPERATIONAL AT ALL TIMES UNTIL THE BASIN'S DESIGN CATCHMENT ACHIEVES 70% GROUND COVER OR SURFACE STABILISATION ACCEPTABLE TO COUNCIL.
 72. THE ESC MEASURES INSTALLED DURING THE DECOMMISSIONING AND REHABILITATION OF A SEDIMENT BASIN SHALL COMPLY WITH SAME STANDARDS SPECIFIED FOR THE NORMAL CONSTRUCTION WORKS.
 73. A SEDIMENT BASIN SHALL NOT BE DECOMMISSIONED UNTIL ALL UP-SLOPE SITE STABILISATION MEASURES HAVE BEEN IMPLEMENTED AND ARE APPROPRIATELY WORKING TO CONTROL SOIL EROSION AND SEDIMENT RUNOFF..
 74. IMMEDIATELY PRIOR TO THE CONSTRUCTION OF THE PERMANENT STORMWATER TREATMENT DEVICE, APPROPRIATE FLOW BYPASS CONDITIONS SHALL BE ESTABLISHED TO PREVENT SEDIMENT-LADEN WATER ENTERING THE DEVICE.
- REVEGETATION/STABILISATION**
75. TEMPORARY STABILISATION MAY BE ATTAINED USING VEGETATION, NON REWETTABLE SOIL POLYMERS, OR PNEUMATICALLY APPLIED EROSION CONTROLS.
 76. ALL CUT AND FILL EARTH BATTERS LESS THAN 3M IN ELEVATION SHALL BE TOPSOILED, AND GRASS SEEDED/HYDROMULCHED WITHIN 10 DAYS OF COMPLETION OF GRADING IN CONSULTATION WITH COUNCIL.
 77. ONCE CUT/FILL OPERATIONS HAVE BEEN FINALISED IN A SECTION, ALL DISTURBED AREAS THAT ARE NOT BEING WORKED ON SHALL BE STABILISED IN ACCORDANCE WITH TIME LINES IN THE BLUE BOOK.
 78. THE LMCC SEED MIX SHALL BE USED UNLESS STATED ON THE ESCP/SWMP.
 79. THE PH LEVEL OF TOPSOIL SHALL BE APPROPRIATE TO ENABLE ESTABLISHMENT AND GROWTH OF SPECIFIED VEGETATION PRIOR TO INITIATING THE ESTABLISHMENT OF VEGETATION.
 80. NON REWETTABLE BINDER SHALL BE USED IN ALL HYDROMULCH/HYDROSEED/POLYMER MIXES ON SLOPES OR WORKS ADJACENT TO A WATER COURSE.
 81. SOIL AMELIORANTS SHALL BE ADDED TO THE SOIL IN ACCORDANCE WITH AN APPROVED LANDSCAPE PLAN, VEGETATION MANAGEMENT PLAN, AND/OR SOIL ANALYSIS.
 82. SURFACE SOIL DENSITY, COMPACTION AND SURFACE ROUGHNESS SHALL BE ADJUSTED PRIOR TO SEEDING/PLANTING IN ACCORDANCE WITH AN APPROVED LANDSCAPE PLAN, VEGETATION MANAGEMENT PLAN, AND/OR SOIL ANALYSIS.
 83. PROCEDURES FOR INITIATING A SITE SHUTDOWN, WHETHER PROGRAMMED OR UN-PROGRAMMED, SHALL INCORPORATE REVEGETATION OF ALL SOIL DISTURBANCES UNLESS OTHERWISE APPROVED BY COUNCIL. THE STABILISATION WORKS SHALL NOT RELY UPON THE LONGEVITY OF NON-VEGETATED EROSION CONTROL BLANKETS, OR TEMPORARY SOIL BINDERS.
- SITE MONITORING AND MAINTENANCE**
84. THE APPLICANT SHALL ENSURE THAT APPROPRIATE PROCEDURES AND SUITABLY QUALIFIED PERSONNEL ARE ENGAGED TO PLAN AND CONDUCT SITE INSPECTIONS AND WATER QUALITY MONITORING THROUGHOUT THE CONSTRUCTION AND MAINTENANCE PHASE.
 85. ALL ESC MEASURES SHALL BE INSPECTED AND ANY MAINTENANCE UNDERTAKEN IMMEDIATELY:
 - a) AT LEAST DAILY (WHEN WORK IS OCCURRING ON-SITE); AND
 - b) AT LEAST WEEKLY (WHEN WORK IS NOT OCCURRING ON-SITE); AND
 - c) WITHIN 24HRS OF EXPECTED RAINFALL; AND
 - d) WITHIN 18HRS OF A RAINFALL EVENT THAT CAUSES RUNOFF ON THE SITE.
 86. WRITTEN RECORDS SHALL BE KEPT ONSITE OF ESC MONITORING AND MAINTENANCE ACTIVITIES CONDUCTED DURING THE CONSTRUCTION AND MAINTENANCE PERIODS, AND BE AVAILABLE TO COUNCIL OFFICERS ON REQUEST.
 87. ALL ENVIRONMENTALLY RELEVANT INCIDENTS SHALL BE RECORDED IN A FIELD LOG THAT SHALL REMAIN ACCESSIBLE TO ALL RELEVANT REGULATORY AUTHORITIES.
 88. ALL WATER QUALITY DATA, INCLUDING DATES OF RAINFALL, DATES OF TESTING, TESTING RESULTS AND DATES OF WATER RELEASE, SHALL BE KEPT IN AN ON-SITE REGISTER. THE REGISTER IS TO BE MAINTAINED UP TO DATE FOR THE DURATION OF THE APPROVED WORKS AND BE AVAILABLE ON-SITE FOR INSPECTION BY [INSERT NAME OF REGULATORY AUTHORITY] ON REQUEST.
 89. AT NOMINATED INSTREAM WATER MONITORING SITES, A MINIMUM OF 3 WATER SAMPLES SHALL BE TAKEN AND ANALYSED, AND THE AVERAGE RESULT USED TO DETERMINE QUALITY.
- INSTREAM WORKS**
90. ALL INSTREAM WORKS (INCLUDING IN OR ADJACENT TO WATERCOURSES NATURAL OR MANMADE, FLOWING OR NOT) SHALL BE CARRIED OUT IN ACCORDANCE WITH THE IECA WHITE BOOKS.

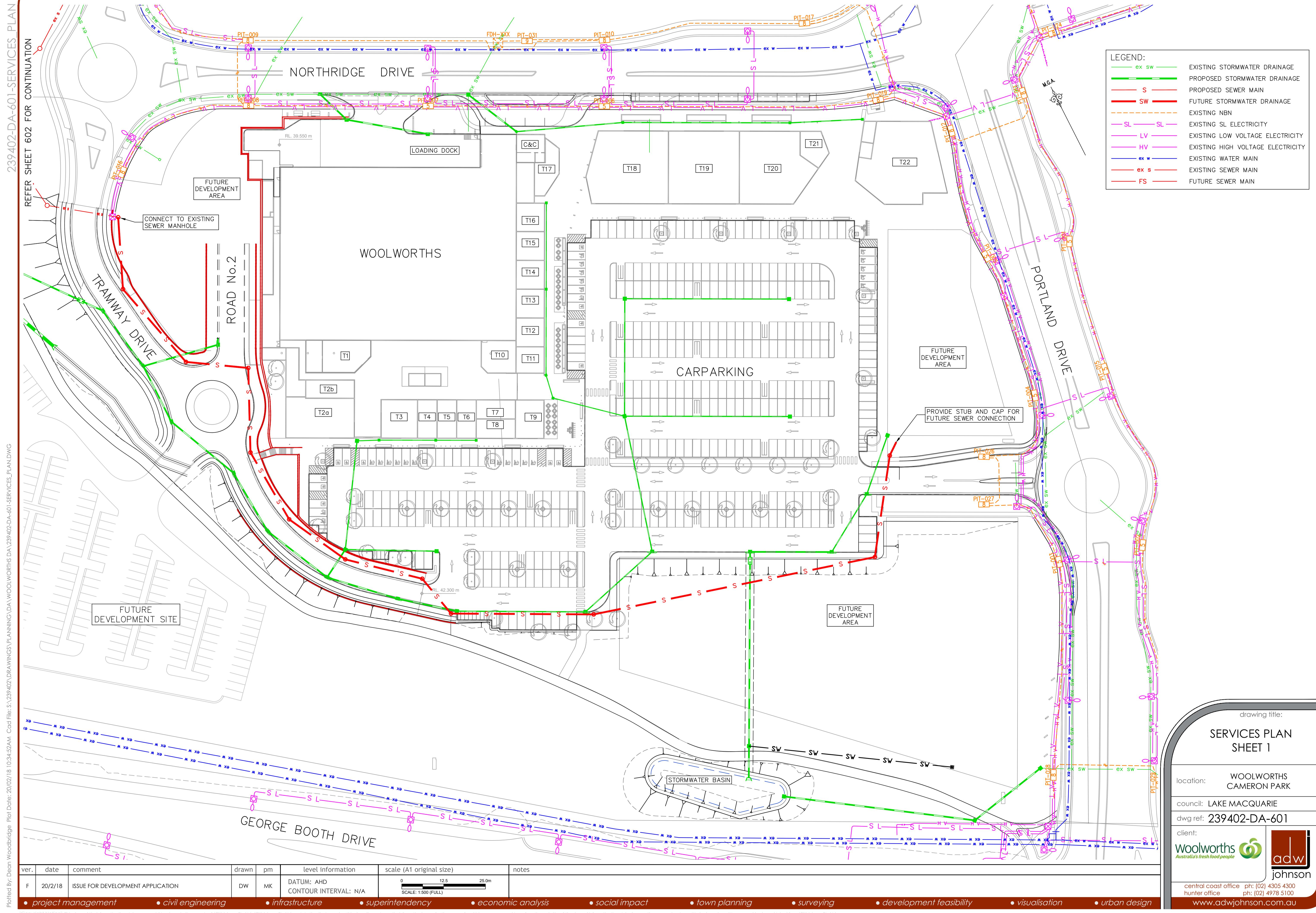


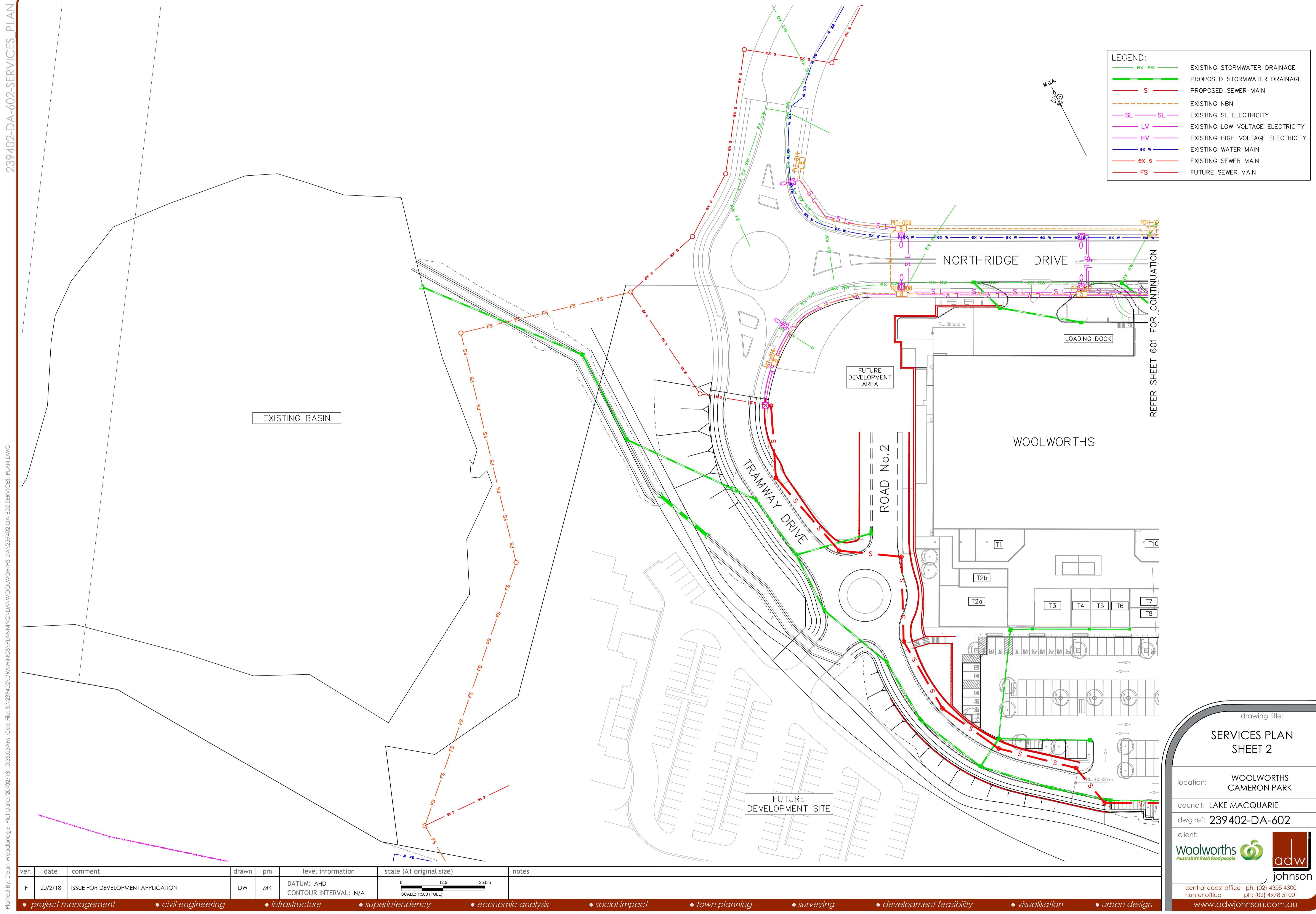
NOTE:
1. ALL LANDSCAPING AND PLANTING
SHALL BE UNDERTAKEN IN
ACCORDANCE WITH ARCHITECTURAL
DOCUMENTATION PACKAGE.

TYPICAL SECTION THROUGH TIERED LANDSCAPE WALL

SCALE 1:25

ver.	date	comment	drawn	pm	level information	scale (A1 original size)	notes
E	16/2/18	ISSUE FOR DEVELOPMENT APPLICATION	DW	MK	DATUM: N/A CONTOUR INTERVAL: N/A	0 3.0 6.0 7.5m SCALE: 1:150 (FULL)	0 0.25 0.50 0.75 1.00 1.25m SCALE: 1:25 (FULL)
		• project management • civil engineering • infrastructure • superintendancy • economic analysis • social impact • town planning • surveying • development feasibility • visualisation • urban design					





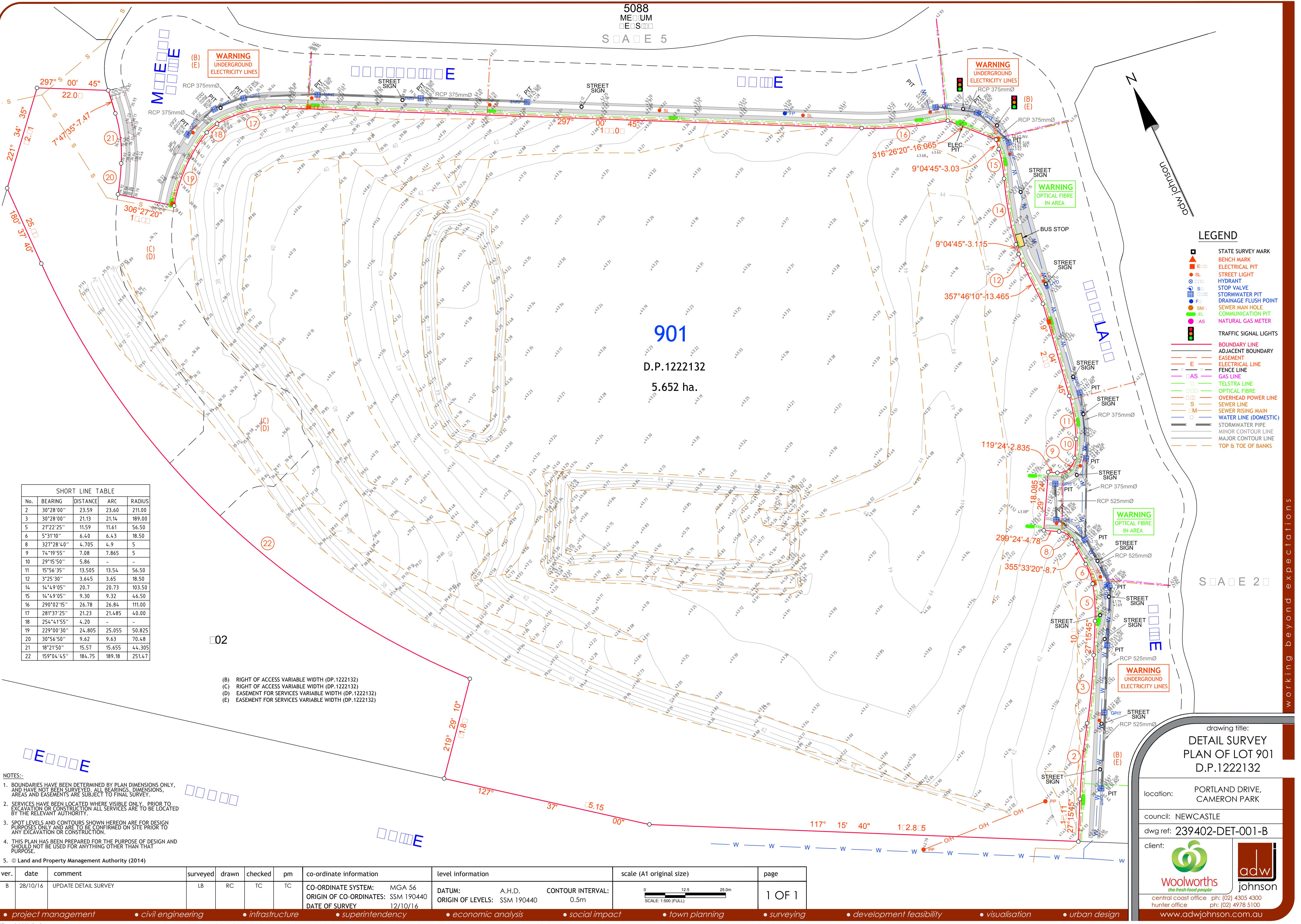


Appendix C

EXISTING SITE

SHORT LINE TABLE			
No.	BEARING	DISTANCE	ARC
2	30°28'00"	23.59	23.60
3	30°28'00"	21.13	21.14
5	21°22'25"	11.59	11.61
6	5°31'10"	6.40	6.43
8	32°28'40"	4.705	4.9
9	74°19'55"	7.08	7.865
10	29°15'50"	5.86	-
11	15°56'35"	13.505	13.54
12	3°25'30"	3.645	3.65
14	14°49'05"	20.7	20.73
15	14°49'05"	9.30	9.32
16	290°02'15"	26.78	26.84
17	281°37'25"	21.23	21.485
18	254°41'55"	4.20	-
19	229°00'30"	24.805	25.055
20	30°56'50"	9.62	9.63
21	18°21'50"	15.57	15.655
22	159°04'45"	184.75	189.18
			251.47

(B) RIGHT OF ACCESS VARIABLE WIDTH (DP.1222132)
 (C) RIGHT OF ACCESS VARIABLE WIDTH (DP.1222132)
 (D) EASEMENT FOR SERVICES VARIABLE WIDTH (DP.1222132)
 (E) EASEMENT FOR SERVICES VARIABLE WIDTH (DP.1222132)



Appendix D

SEDIMENT BASIN SIZING CALCULATIONS

Revised Universal Soil Loss Equation (RUSLE) – BASIN 1

Catchment Area (A)	3	ha	
Settling Zone			
Runoff Coefficient (Cv)	0.64		
85th %, 5 day Rainfall Event (R)	38.9		Table 6.3a, pg 6-24
Settling Zone Volume	746.88	m³	
Sediment Zone			
Disturbed Site Area (hectares)	3	ha	
Rainfall Erosivity Factor, R	2578		
Soil Erodibility Factor, K	0.07		Table C17
Slope Length Gradient Factor, LS	0.52	Approx 150m@2%	Table A1, pg A-9
Erosion Control Practice Factor, P	1.3		Table A2, pg A-11
Cover Factor, C	1		Figure A5, pg A-12
Sediment Zone Volume	48	m³	
Total Storage Required (Settling + Sediment + 10%)	874	m³	

Revised Universal Soil Loss Equation (RUSLE) – BASIN 2

Catchment Area (A)	1.25	ha	
Settling Zone			
Runoff Coefficient (Cv)	0.64		
75th %, 5 day Rainfall Event (R)	38.9		Table 6.3a, pg 6-24
Settling Zone Volume	311.2	m³	
Sediment Zone			
Disturbed Site Area (hectares)	1.25	ha	
Rainfall Erosivity Factor, R	2578		
Soil Erodibility Factor, K	0.07		Table C17
Slope Length Gradient Factor, LS	0.52	Approx 150m@2%	Table A1, pg A-9
Erosion Control Practice Factor, P	1.3		Table A2, pg A-11
Cover Factor, C	1		Figure A5, pg A-12
Sediment Zone Volume	20	m³	
Total Storage Required (Settling + Sediment + 10%)	364	m³	



Appendix E

BASIN MAINTENANCE CHECKLIST

WOOLWORTHS, CAMERON PARK – MAINTENANCE FORM					
Location	Cameron park				
Description	Constructed ponds and associated infrastructure				
SITE VISIT DETAILS					
Site visit date:					
Site visit by:					
Weather:					
Purpose of the site visit	Tick Box		Complete Sections		
Routine inspection	0		Section 1 only		
Routine maintenance	0		Section 1 and 2		
Cleanout of sediment	0		Section 1, 2 and 3		
Annual inspection	0		Section 1, 2, 3 and 4		
SECTION 1 - INSPECTION					
Gross pollutant load cleanout required?	Yes/No - If yes please see Ecosol Maintenance Schedule				
Depth of sediment in forebay:	m				
Cleanout required if depth of sediment >1.0m	Yes/No				
Any weeds or litter in basin (if Yes, complete Section 2 – Maintenance)	Yes No				
Any visible damage to basin? (If Yes, complete Section 4 – Condition)	Yes/No				
Inspection comments:					
SECTION 2 – GENERAL MAINTENANCE					
Were the weeds removed this site visit?	Yes/No				
Is there litter in the lake or forebay?	Yes/No				
Was the litter collected this site visit?	Yes/No				
SECTION 3a – CLEANOUT OF GROSS POLLUTANTS					
Have the following been notified of cleanout date?	Yes No				
Coordinator – open space and/or drainage	0 0				
Local residents	0 0				
Other (specify)	0 0				
Method of cleaning (excavator or eductor)					
Volume of gross pollutant and sediment removed (approximate estimate)	m ³				
Any visible damage to wetland or SW tailouts? (If yes, complete Section 4 – Condition)	Yes/No				
SECTION 3b – CLEANOUT OF SEDIMENT					
Have the following been notified of cleanout date?	Yes No				
Coordinator – open space and/or drainage	0 0				
Local residents	0 0				
Other (specify.....)	0 0				
Method of cleaning (excavator or educator)					
Volume of sediment removed (approximate estimate)	m ³				
Any visible damage to wetland or sediment forebay? (If yes, complete Section 4 – Condition)	Yes/No				
SECTION 4 – CONDITION ASSESSMENT					
Component	Checked?		Condition OK?		Remarks
	Yes	No	Yes	No	
Inlet structures					
Outlet structures					
Spillway and spillway channel					
Littoral zone vegetation					
Banks and batter slopes					
Retarding basin embankment					
Surrounding landscaping					
Comments:					

Appendix F

EXTRACT FROM "CAMERON GROVE" PLANS, BROWNS CONSULTING

FOR CONTINUATION REFER TO DRAWING 10

E:
- BULK EARTHWORKS TO BE
CARRIED OUT BY OTHERS PRIOR

- REFER MARKET PLACE DESIGN
BY MOTT MACDONALD DWG. No.
- 295891C - FWC01

CONSTRUCT BUS SHELTERS TO
COUNCIL STANDARD DWG'S.

MARKET PLACE

STAGE 2

GEORGE

BOOTH

DR

TRANSITION EXISTING 2.5m SH
PATH TO 1.2m WIDE CONCRETE
FOOTPATH TO COUNCIL STAND

LIMIT OF CONSTRUCTION AT W.A.D. CH 41.841. MAKE SMOOTH CONNECTION TO EXISTING PAVEMENT.

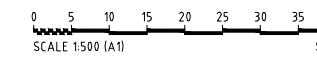
REFER TO GEORGE BOOTH DRIVE INTERSECT
DESIGN PREPARED BY BROWN CONSULTING.
REF: X08116_01 DATED: 08/06/2012

The diagram illustrates a transition from a 2.5m wide shared path to a 2m wide concrete footpath. The shared path is indicated by a dashed line with diagonal hatching. A solid line with diagonal hatching indicates the new 2m wide concrete footpath. A vertical dashed line marks the transition point. An angle of 85°25' is shown between the two paths at the transition point.

FOR CONSTRUCTION CERTIFICATE

	00	AW	DD	CB	TT	12/04/13	ISSUED FOR 80% REVIEW
Revisions	01	AW	DD	CB	SG	2/10/13	ISSUED FOR CONSTRUCTION
	02	VP	JP	SG		16/07/14	ISSUED WITH COUNCIL AMEND.
	03	BL	BL	PF	SAG	25/05/15	UPDATED TO COUNCIL COMME
	04	BL	BL	PF	SAG	5/08/15	NOTE ADDED TO RIGID PAVEM
	05	BL	BL	PF		02/09/15	UPDATED TO REFLECT COMME
First Issue	AW	DD	CB	TT	12/04/13		
	Drawn	Design	Check	Appd.	Date		

CERTIFICATE
MENTS
ENTS
ENT PLAN REGARDING SURFACE FINISH
ENTS BY GEOTECHNICAL ENGINEER



50
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Approval:
BY: STUART GREEN
FIEAust. CPEng.
Manager - Urban Developm
SIGN: 

Client: HAMMERSMITH MANAGEMENT PTY LTD
Project: CAMERON GROVE - LINK ROAD
ROAD & DRAINAGE DESIGN

BROWN
Smart Consulting

Drawing Title:
CIVIL ENGINEERING PLAN
SHEET 01 OF 03

Project No.: Stage: Milestone: Dwg No.: Revis
L05016.014 CC 101 05

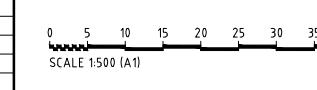
FOR CONTINUATION REFER TO DRAWING 103

Revisions	00
01	01
02	02
03	03
04	04
05	05



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	03	BL	BL	PF	SAG	25/05/15	UPDATED TO COUNCIL COMMENTS
	04	BL	BL	PF	SAG	5/08/15	NOTE ADDED TO RIGID PAVEMENT PLAN REGARDING SURFACE FINISH
	05	BL	BL	PF		2/09/15	UPDATED TO REFLECT COMMENTS BY GEOTECHNICAL ENGINEER
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Manager - Urban Develop
SIGN:

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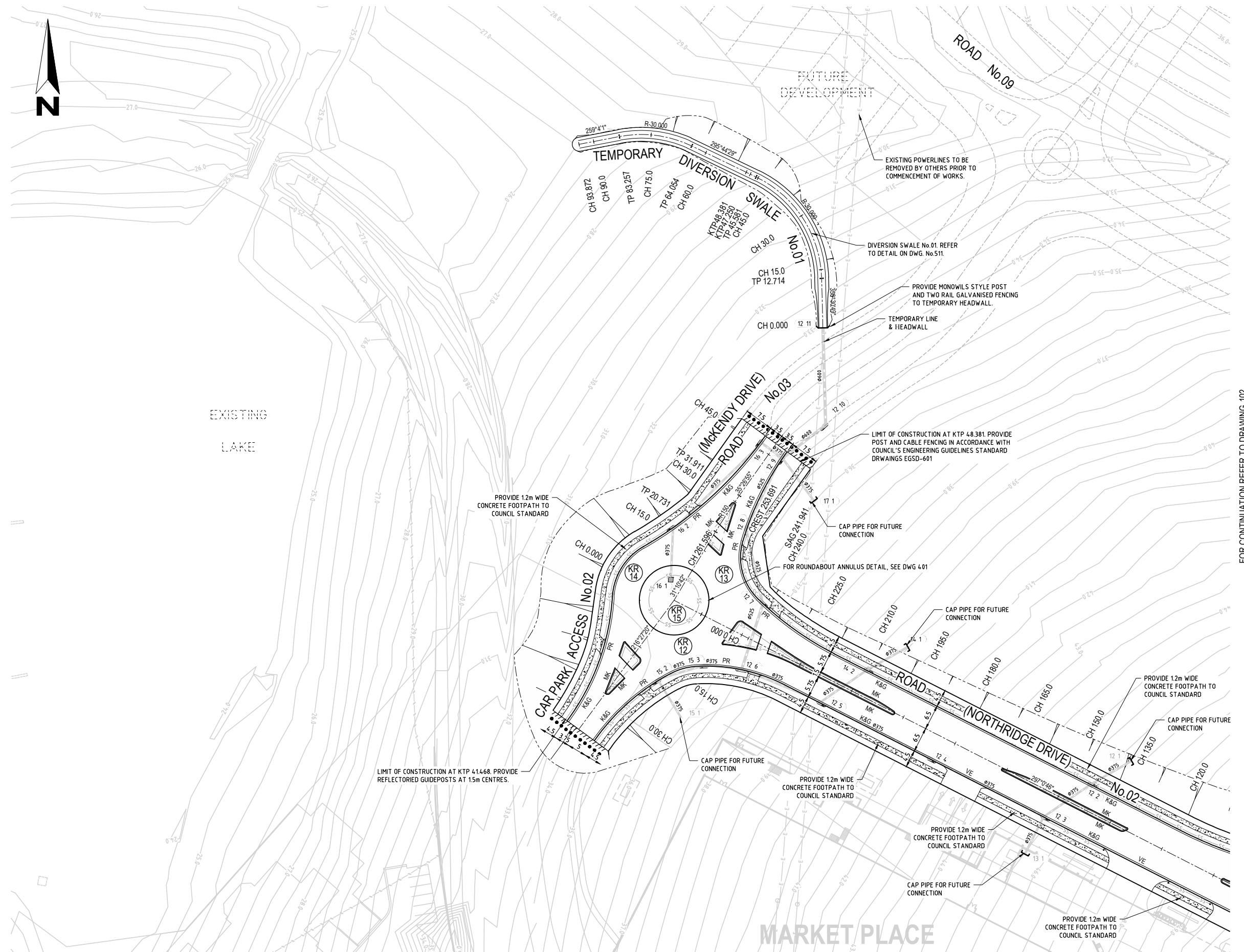
Project: CAMERON GROVE - LINK ROAD

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Project No.:	Stage:	Milestone:	Dwg No.:	Revision:
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E:
 - BULK EARTHWORKS TO BE
 CARRIED OUT BY OTHERS PRIOR
 TO COMMENCEMENT OF WORKS.
 - REFER MARKET PLACE DESIGN
 BY MOTT MACDONALD DWG. NO.
 295891C-EWC01.



MARKET PLACE

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SCALE 1:500 (A1) SCALE 1:1000 (A3)

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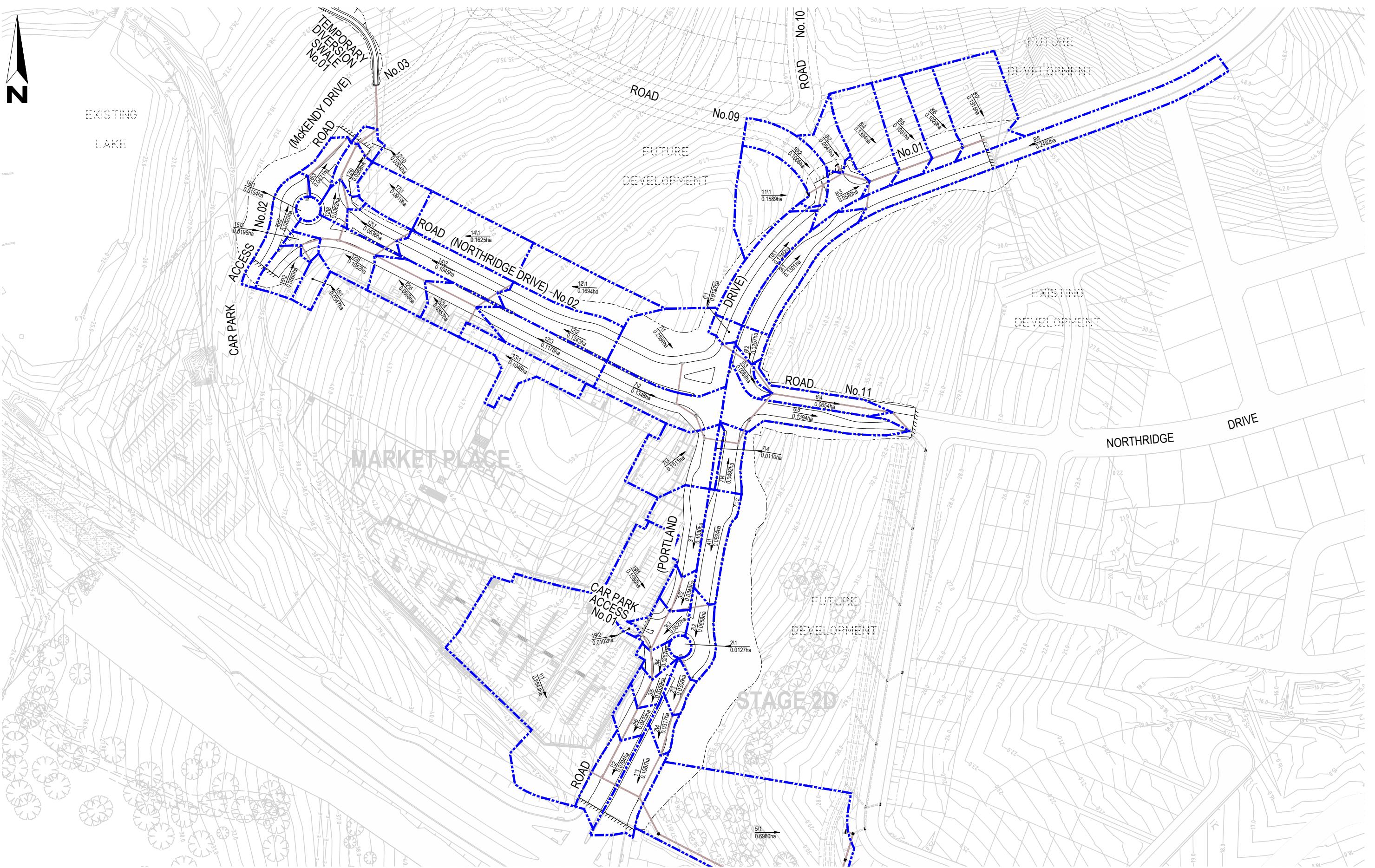
Approval:
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 Manager - Urban Development
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 DATE:

Client:
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 Project:
CAMERON GROVE - LINK ROAD
ROAD & DRAINAGE DESIGN

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Drawing Title:
CIVIL ENGINEERING PLAN
SHEET 03 OF 03

Project No.: L05016.014 Stage: Milestone: Dwg No.: Revision:
 CC 103 05



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FILE H:\\L05016 - R001.DWG - REV 014 DATE 01/04/13 L05016.R001.DWG DRAWN BY: S. LAWRENCE CHECKED BY: J. LAWRENCE APPROVED BY: S. LAWRENCE DRAWN ON BEHALF OF: BROWN CONSULTING PTY LTD

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Drawn	Design	Check	Appd.	Date		

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DATE: 

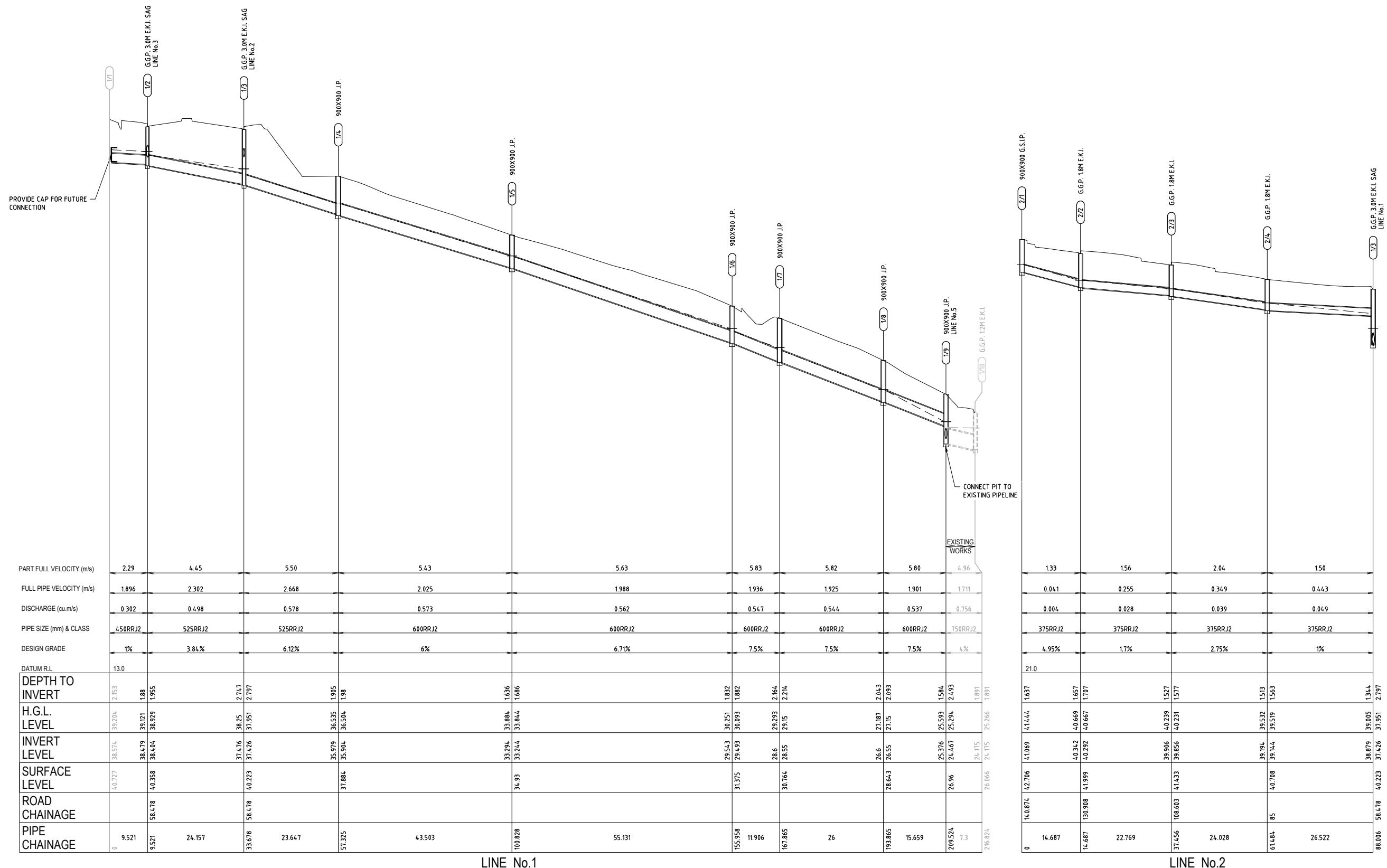
Client:
HAMMERSMITH MANAGEMENT PTY LTD
Project:
CAMERON GROVE - LINK ROAD
ROAD & DRAINAGE DESIGN

BROWN
Smart Consulting

Drawing Title:
CATCHMENT PLAN
Project No.: L05016.014 Stage: CC Milestone: 501 Revision: 05

NOTE:

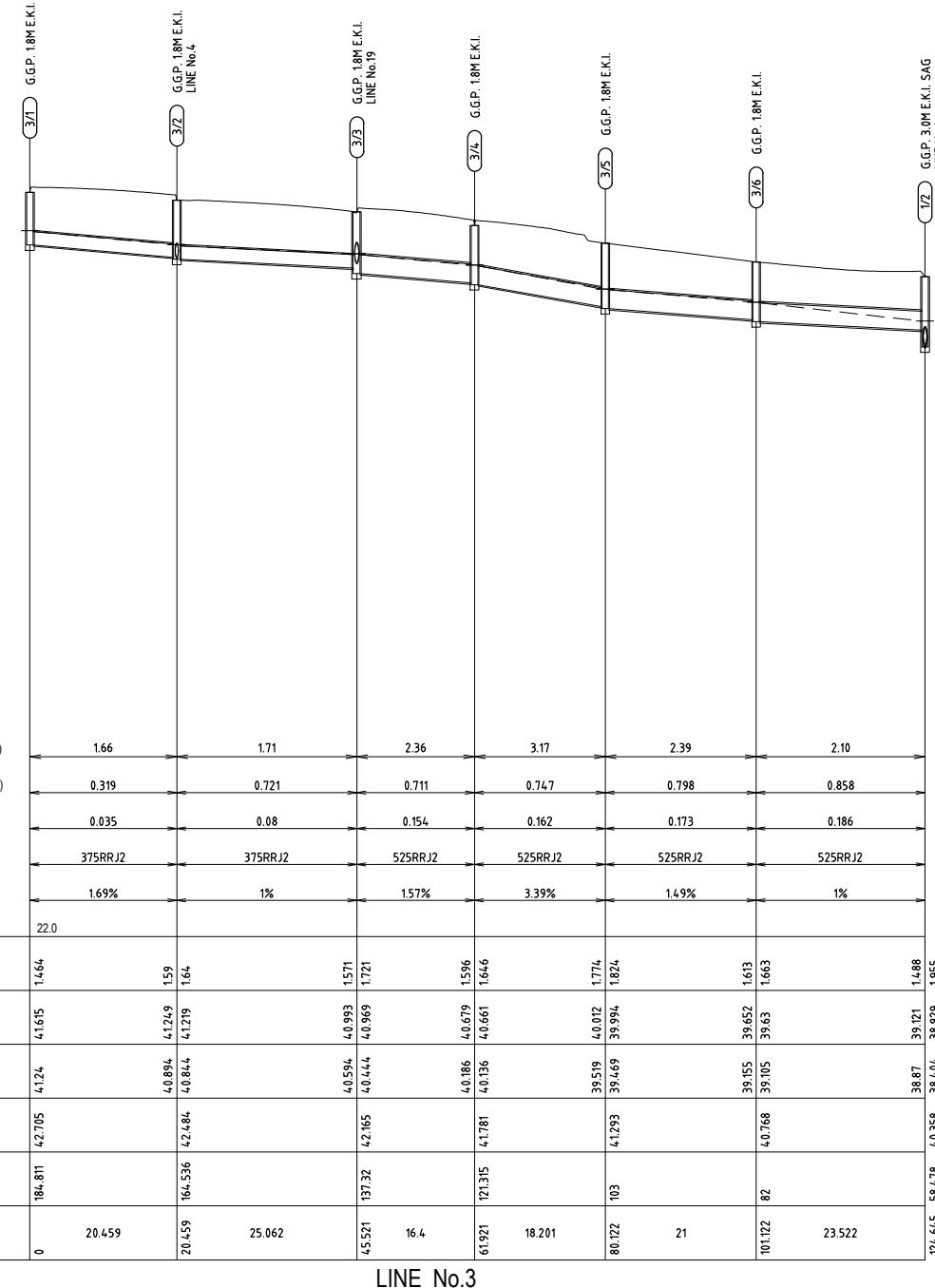
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FOR CONSTRUCTION CERTIFICATE



NOTE:
- ALL DRAINAGE PITS TO BE TO COUNCIL STANDARDS UNLESS OTHERWISE SPECIFIED.



PART FULL VELOCITY (m/s)

1.66

1.71

2.36

3.17

2.39

2.10

FULL PIPE VELOCITY (m/s)

0.319

0.721

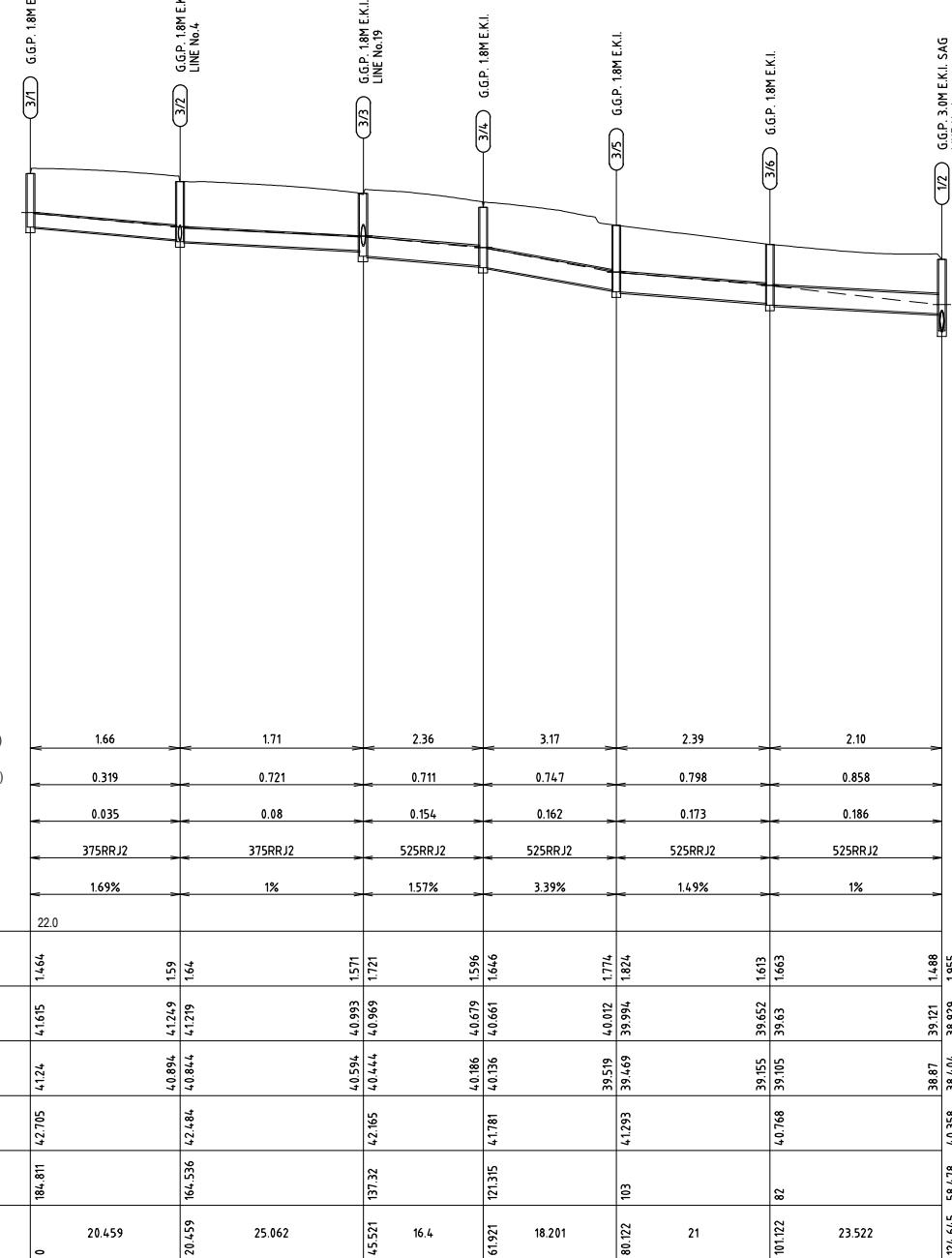
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0.747

0.798

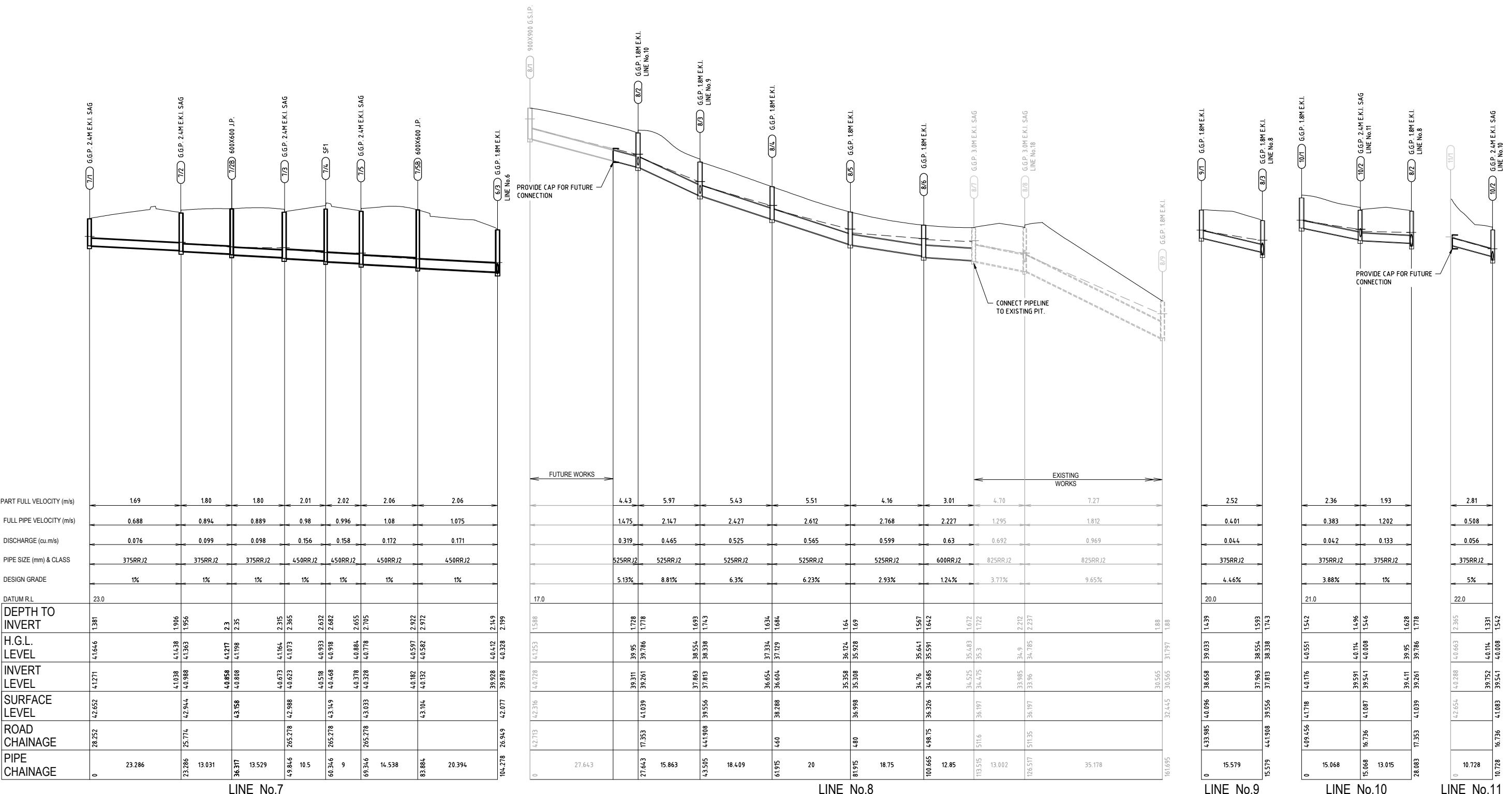
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LINE No.3



NOTE:

- ALL DRAINAGE PITS TO BE TO COUNCIL STANDARDS UNLESS OTHERWISE SPECIFIED.



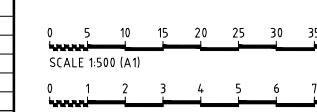
FOR CONSTRUCTION CERTIFICATE



Drawing Title:
DRAINAGE LONGITUDINAL SECTIONS
LINE No.7-11

Project No.: Stage: Milestone: Dwg No.: Revision:
L05016.014 CC 504 05

PARKER LONGWELL LTD. LTD. LK1							
Revisions	00	AW	DD	CB	TT	12/04/13	ISSUED FOR 80% REVIEW
	01	AW	DD	CB	SG	2/10/13	ISSUED FOR CONSTRUCTION CERTIFICATE
	02	VP	JP	SG		16/07/14	ISSUED WITH COUNCIL AMENDMENTS
	03	BL	BL	PF	SAG	25/05/15	UPDATED TO COUNCIL COMMENTS
	04	BL	BL	PF	SAG	5/08/15	NOTE ADDED TO RIGID PAVEMENT PLAN REGARDING SURFACE FINISH
	05	BL	BL	PF		02/09/15	UPDATED TO REFLECT COMMENTS BY GEOTECHNICAL ENGINEER
	First Issue	AW	DD	CB	TT	12/04/13	
Drawn	Design Check	Approved	Appd.	Date			Revision Details



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 SCALE.

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 SCALE 1:1000 (A3)

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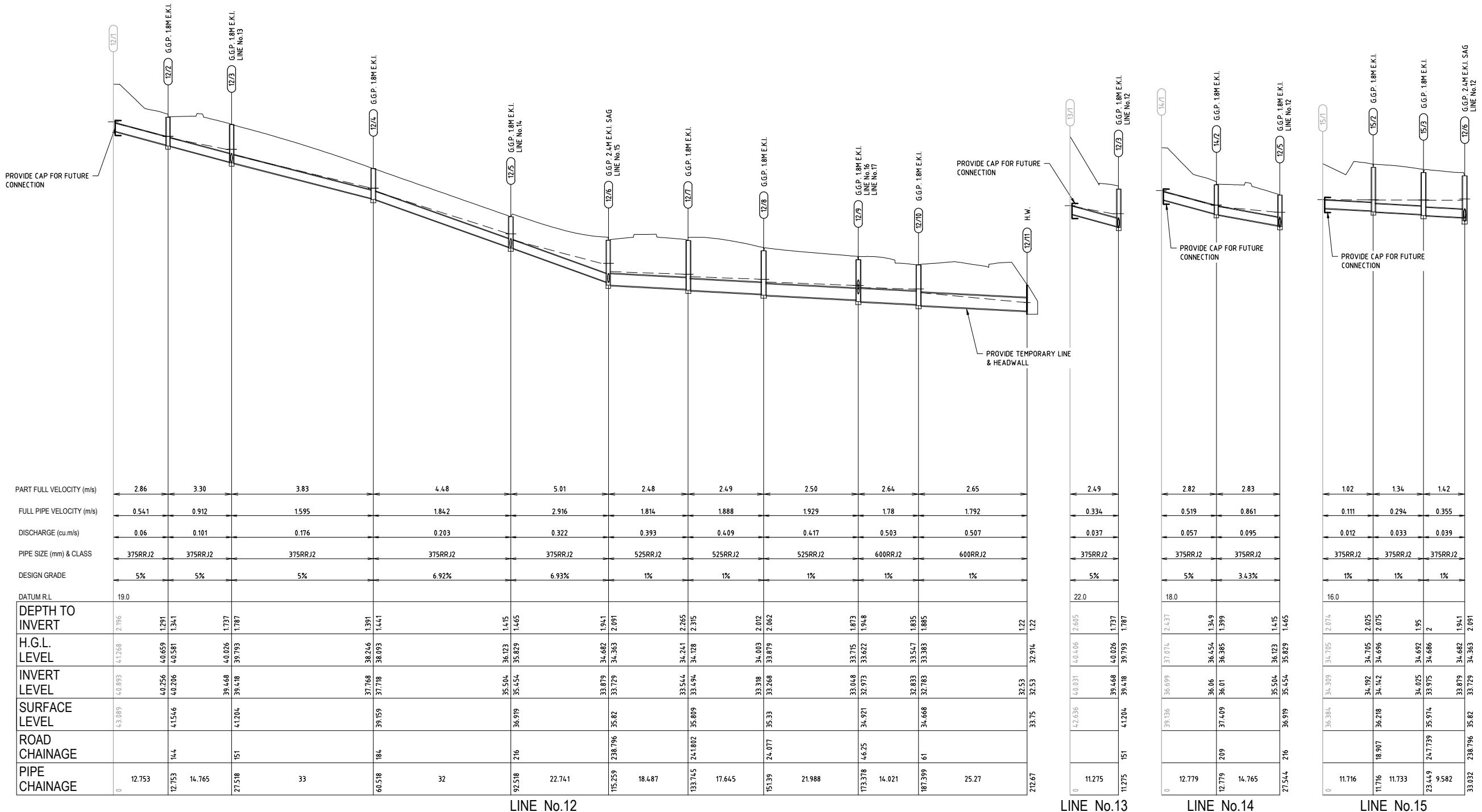


Approval:
BY: STUART GREEN
FIEAust, CPEng.
Manager - Urban Developments
SIGN: 

Client:
HAMMERSMITH MANAGEMENT PTY LTD

Project:
**CAMERON GROVE - LINK ROAD
ROAD & DRAINAGE DESIGN**

NOTE:
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LINE No.12

LINE No.13

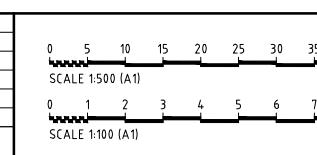
LINE No.14

LINE No.15

FOR CONSTRUCTION CERTIFICATE

ISSUED FOR 80% REVIEW									
Revisions	00	AW	DD	CB	TT	12/04/13	ISSUED FOR CONSTRUCTION CERTIFICATE		
01	AW	DD	CB	SG	2/10/13	12/04/13	ISSUED FOR CONSTRUCTION CERTIFICATE		
02	VP	JP	SG		16/07/14		ISSUED WITH COUNCIL AMENDMENTS		
03	BL	BL	PF	SAG	25/05/15		UPDATED TO COUNCIL COMMENTS		
04	BL	BL	PF	SAG	5/08/15		NOTE ADDED TO RIGID PAVEMENT PLAN REGARDING SURFACE FINISH		
05	BL	BL	PF		02/09/15		UPDATED TO REFLECT COMMENTS BY GEOTECHNICAL ENGINEER		
First Issue	AW	DD	CB	TT	12/04/13				
	Drawn	Design	Check	Appd.	Date				

Revision Details



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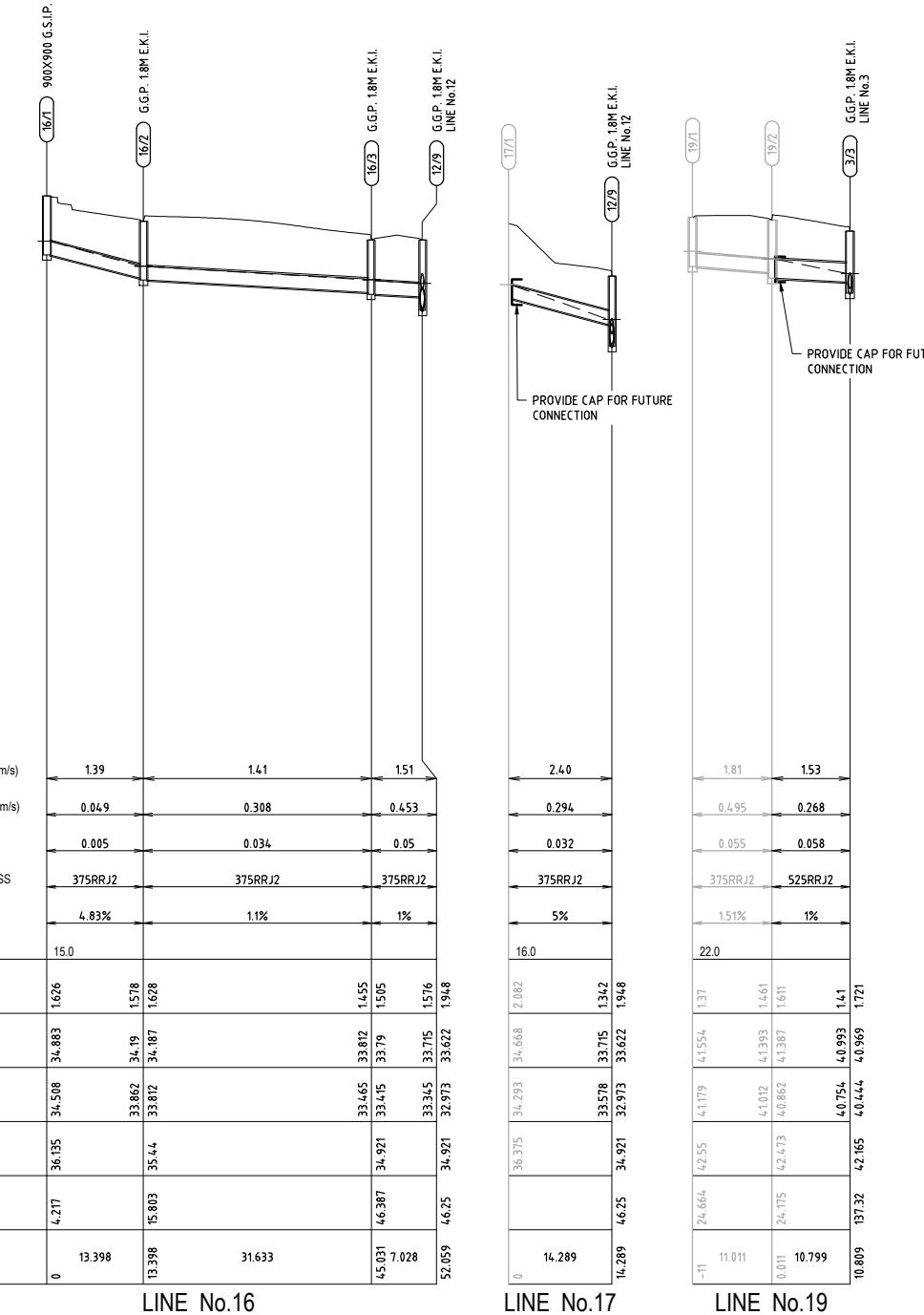
Approval:
BY: STUART GREEN
FIEAust, CPEng,
Manager - Urban Development
SIGN:

Client:
HAMMERSMITH MANAGEMENT PTY LTD
Project:
CAMERON GROVE - LINK ROAD
ROAD & DRAINAGE DESIGN

BROWN
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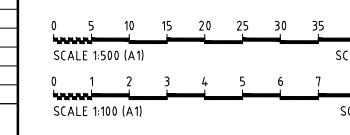
Drawing Title:
DRAINAGE LONGITUDINAL SECTIONS
LINE No.12-15
Project No.: L05016.014 Stage: Milestone: Dwg No.: Revision: CC 505 05

NOTE:
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FOR CONSTRUCTION CERTIFICATE

Revisions	00	AW	DD	CB	TT	12/04/13	ISSUED FOR 80% REVIEW
01	AW	DD	CB	SG	2/10/13	ISSUED FOR CONSTRUCTION CERTIFICATE	
02	VP	JP	SG		16/07/14	ISSUED WITH COUNCIL AMENDMENTS	
03	BL	BL	PF	SAG	25/05/15	UPDATED TO COUNCIL COMMENTS	
04	BL	BL	PF	SAG	5/08/15	NOTE ADDED TO RIGID PAVEMENT PLAN REGARDING SURFACE FINISH	
05	BL	BL	PF		02/09/15	UPDATED TO REFLECT COMMENTS BY GEOTECHNICAL ENGINEER	
First Issue	AW	DD	CB	TT	12/04/13		
	Drawn	Design	Check	Appd.	Date		
	Revision Details						



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Client:
HAMMERSMITH MANAGEMENT PTY LTD
Project:
CAMERON GROVE - LINK ROAD
ROAD & DRAINAGE DESIGN

BROWN
Smart Consulting

Drawing Title:
DRAINAGE LONGITUDINAL SECTIONS
LINE No.16, 17 & 19
Project No.: L05016.014 Stage: CC Milestone: 506 Revision: 05

12D MODEL - HYDROLOGICAL DESIGN SHEET

MINOR 10 YEAR STORM EVENT

Pit Name	Pit Type	Setout Easting (m)	Setout Northing (m)	RL (m)	RL (m)	Road Name	Road Chaining (m)	Catch ID	Time Tc (min)	Intensity I (-)	Runoff C (-)	Area A (ha)	Full CA (ha)	Full Sum CA (ha)	Partial Q=CIA (L/s)	Partial Sum CA (ha)	Partial Q=CIA (L/s)	Partial Flow Qc (L/s)	Approach Road Capacity (L/s)	Flooded Depth (m)	Flooded Width (m)	Flooded Vel.Dep (sq.m/s)	Road Grade (%)	Road Xfall (%)	Road Depth (m)	Max Pond Inlet Flow (L/s)	Bypass Flow Qb (L/s)	Pit (-)		
1\1	900x900 G.S.I.P.	368905.23	6356950.12	40.73	40.73			1P	6	149	0.43	0.0854	0.0365	0.7286	301.5	0.0365	0.7286	301.5	301.5	301.5	301.5	301.5	0.6921	-	1\2					
1\2	G.G.P. 3.0m E.K.I. SAG	368914.04	6356945.38	40.36	40.36	ROAD No 1	58.48	1P	6	149	0.43	0.0079	0.0034	0.0677	28	0.0034	0.0677	28	28	28	510.1	0.031	-	0.18	28	1\3				
1\3	G.G.P. 3.0m E.K.I. SAG	368934.66	6356934.75	40.22	40.22	ROAD No 1	58.48	1P	6	149	0.43	0.0109	0.0046	0.0927	38.4	0.0046	0.0927	38.4	38.4	38.4	488.4	0.038	-	0.175	38.4	LOST				
1\4	900x900 J.P.	368946.03	6356931.57	37.88	37.88						0.43	0.0979	0.0881	0.0881											-					
1\5	900x900 J.P.	368984.71	6356933.66	34.93	34.93						0.43	0.0115	0.0103	0.0103											-					
1\6	900x900 J.P.	369034	6356868.96	31.38	31.38						0.43	0.0066	0.0028	0.0561	23.2	0.0028	0.0561	23.2	23.2	23.2	24.7						-			
1\7	900x900 J.P.	369044.86	6356873.83	30.76	30.76						0.43	0.0052	0.0038	0.0533	0.0533											-				
1\8	900x900 J.P.	369052.36	6356898.72	28.64	28.64						0.43	0.0109	0.0046	0.0927	38.4	0.0046	0.0927	38.4	38.4	38.4	488.4	0.038	-	0.175	38.4	LOST				
1\9	900x900 J.P.	369056.88	6356933.72	26.96	26.96						0.43	0.0979	0.0881	0.0881											-					
1\10	G.G.P. 1.2m E.K.I.	369063.89	6356936.49	26.07	26.07						0.43	0.0079	0.0046	0.0881											-					
2\1	900x900 G.S.I.P.	368957.92	6357015.41	42.53	42.53	ROAD No 1	140.87	1P	6	149	0.43	0.0013	0.0005	0.0109	4.5	0.0005	0.0109	4.5	4.5	4.5	4.5				48.8		4.5	2\2		
2\2	G.G.P. 1.8m E.K.I.	368963.91	6357001.77	42	42	ROAD No 1	130.91	1P	6	149	0.43	0.0066	0.0028	0.0561	23.2	0.0028	0.0561	23.2	23.2	23.2	24.7				1.8	1.9	24.3	0.4	2\3	
2\3	G.G.P. 1.8m E.K.I.	368953.62	6356981.37	41.43	41.43	ROAD No 1	108.6	1P	6	149	0.43	0.0031	0.0013	0.0261	10.8	0.0013	0.0261	10.8	10.8	11.2	2110.8	0.05	0.79	0.04	2.5	3	11.2	2\4		
2\4	G.G.P. 1.8m E.K.I.	368946.81	6356958.33	40.71	40.71	ROAD No 1	85	1P	6	149	0.43	0.0032	0.0014	0.027	11.2	0.0014	0.027	11.2	11.2	11.2	2077	0.05	0.78	0.04	2.5	3	11.2	1\3		
3\1	G.G.P. 1.8m E.K.I.	368959.87	6357059.46	42.7	42.7	ROAD No 1	184.81	1P	6	149	0.43	0.0103	0.0044	0.0878	36.3	0.0044	0.0878	36.3	36.3	36.3	4779.8	0.081	1.85	0.05	1	3	34	2.4	3\2	
3\2	G.G.P. 1.8m E.K.I.	368952.89	6357040.05	42.48	42.48	ROAD No 1	164.54	1P	6	149	0.43	0.0035	0.0015	0.0298	12.3	0.0015	0.0298	12.3	12.3	14.7	1146.6	0.06	1.58	0.03	1.2	2.1	14.7	3\3		
3\3	G.G.P. 1.8m E.K.I.	368942.03	6357017.79	42.16	42.16	ROAD No 1	137.32	1P	6	149	0.43	0.0028	0.0011	0.0462	19.1	0.0011	0.0462	19.1	19.1	19.1	9050.2	0.062	1.53	0.04	1.7	0.9	19.1	3\4		
3\4	G.G.P. 1.8m E.K.I.	368942.81	6357001.24	41.78	41.78	ROAD No 1	121.31	1P	6	149	0.43	0.0027	0.0011	0.0227	9.4	0.0011	0.0227	9.4	9.4	9.4	7986	0.047	0.67	0.03	2.3	2.4	9.4	3\5		
3\5	G.G.P. 1.8m E.K.I.	368934.43	6356984.96	41.29	41.29	ROAD No 1	103	1P	6	149	0.43	0.0036	0.0015	0.0306	12.7	0.0015	0.0306	12.7	12.7	12.7	11665.4	0.052	0.86	0.04	2.5	3	12.7	3\6		
3\6	G.G.P. 1.8m E.K.I.	368924.81	6356966.29	40.77	40.77	ROAD No 1	82	1P	6	149	0.43	0.0042	0.0018	0.0361	14.9	0.0018	0.0361	14.9	14.9	14.9	2968.4	0.055	0.95	0.04	2.5	3	14.9	1\2		
4\1	G.G.P. 1.8m E.K.I.	368974.24	6357044.19	42.5	42.5	ROAD No 1	172	1P	6	149	0.43	0.0092	0.0039	0.0788	32.6	0.0039	0.0788	32.6	32.6	32.6	1307.3	0.079	1.76	0.05	1	3	31.1	1.5	2\2	
5\1	900x900 G.S.I.P.	369052.09	6356915.16	27.92	27.92			1P	6	149	0.43	0.0698	0.0298	0.0592	245.3	0.0298	0.0592	245.3	245.3	245.3	246.3						246.3	-		
6\1	G.G.P. 1.8m E.K.I.	368982.58	6357192.42	43.84	43.84	ROAD No 1	318.83	1P	6	149	0.43	0.002	0.0008	0.0168	6.9	0.0008	0.0168	6.9	6.9	6.9	3508.1	0.049	0.76	0.02	1	2.9	6.9	7\1		
6\2	G.G.P. 1.8m E.K.I.	368998.58	6357181.01	43.61	43.61	ROAD No 1	311.92	1P	6	149	0.43	0.0026	0.0011	0.0219	9.1	0.0011	0.0219	9.1	9.1	9.1	2210.6	0.049	0.77	0.03	1.6	3	9.1	6\3		
6\3	G.G.P. 1.8m E.K.I.	369010.96	6357163.93	42.08	42.08	ROAD No 11	26.95	1P	6	149	0.43	0.0027	0.0011	0.0228	9.4	0.0011	0.0228	9.4	9.4	9.4	5001	0.035	0.4	0.05	9	3	9.4	6\4		
6\4	G.G.P. 1.8m E.K.I.	369079.22	6357153.23	32.5	32.5	ROAD No 11	96	1P	6	149	0.43	0.0065	0.0028	0.0558	23.1	0.0028	0.0558	23.1	23.1	23.1	1153.2	0.048	0.74	0.08	13.1	3	23.1	LOST		
6\5	G.G.P. 3.0m E.K.I.	369089.21	6357143.47	31.09	31.09																									

12D MODEL - HYDROLOGICAL DESIGN SHEET (cont.)

MINOR 10 YEAR STORM EVENT

FOR CONTINUATION REFER TO DRAWING No.507

NOTE:

ALL DRAINAGE PITS TO BE TO COUNCIL
STANDARDS UNLESS OTHERWISE SPECIFIED

12D MODEL - HYDRAULICS DESIGN SHEET

MINOR 10 YEAR STORM EVENT

Pipe ID	Pipe Type	Pipe Length (m)	Pipe Size (mm)	Pipe Area Af (sq.m)	Pipe Grade (%)	Pipe Tct (min)	Full-area I (mm/hr)	Full-area Sun CA (l/s)	Full-area Qc=CIA (l/s)	Part-area Tct (min)	Part-area I (mm/hr)	Part-area Sum CA (ha)	Part-area Qc=CIA (U/s)	Peak Flow Q (l/s)	Net Bypass Flow Q (l/s)	Pipe Vel Vn/Q (m/s)	Capacity Qcap (m³/s)	Full Pipe Depth (m)	Norm Depth (m)	Crit Depth Vel Vn/Q/A (m)	Capacitv Vel Qcap/Af (m³/s)	US Pit Grate RL (m)	Pipe US IL (m)	Pipe DS IL (m)	DS Pit Grate RL (m)	Cover Limit	Pipe Min (m)	Pipe DS Bend (deg)	Pipe DS Drop (m)	US Pit Pipe Ku (m)	US Pit Pipe Kv (m)	Pipe P'head Loss (Kw/V'head)	WSE Loss (Kw/V'head)	Pipe Head Loss (m)	US Pit HGL (m)	Pipe DS HGL (m)	HGL Grade (%)	HGL Grade (%)					
1 1 to 1 2	RJRJ	9.52	45.0	0.159	1	100	6	149	0.7286	301.5	6	149	0.7286	301.5	301.5	323	0.9	1.9	2.29	2.09	2.03	40.73	38.57	38.48	40.36	1.2	1.1	-1.1	0.075	4.5	0.18	0.83	0.08	40.39	39.12	39.12	0.87	114.6					
1 2 to 1 3	RJRJ	24.66	52.5	0.216	3.84	26	7.06	140.55	1.269	495.5	6.08	148.21	1.2122	499	499	-1.5	497.5	952.5	0.52	2.3	4.44	4.4	40.36	38.4	37.48	40.22	1	1.42	35.2	0.05	0.71	0.27	0.19	0.68	39.12	38.98	38.5	38.25	28.61	35.6			
1 3 to 1 4	RJRJ	23.65	52.5	0.216	6.12	16.3	7.26	138.4	1.4818	571.9	6.61	148.14	1.4659	577.6	577.6	57.6	57.6	120.3	0.48	2.67	5.5	5.56	20.42	37.43	35.98	35.78	1.2	1.77	-3.52	0.075	0.82	0.36	0.3	1.42	38.25	37.95	36.54	36.54	5.99	16.7			
1 4 to 1 5	RJRJ	43.5	60.0	0.283	6	16.7	7.45	137.37	1.4818	565.4	6.93	141.42	1.4578	572.7	572.7	57.2	57.2	169.1	0.34	2.03	5.43	2.3	5.98	37.88	35.9	33.29	34.93	1	1.03	-0.6	0.05	0.15	0.21	0.03	2.62	36.54	36.5	33.88	33.88	0.62	16.6		
1 5 to 1 6	RJRJ	55.13	60.0	0.283	6.71	14.9	7.82	140.47	1.4818	55.5	7.13	139.94	1.4463	562.2	562.2	56.2	56.2	178.2	0.31	1.99	5.63	2.28	6.33	34.99	33.24	29.94	31.34	1	1.01	-50.7	0.05	0.2	0.04	3.59	33.88	33.84	30.25	30.62	6.52	15.3			
1 6 to 1 7	RJRJ	11.91	60.0	0.283	7.5	13.3	8.28	131.34	1.4818	540.6	7.44	17.48	1.4330	547.5	547.5	54.7	54.7	189.1	0.29	1.94	5.83	2.24	6.69	31.38	29.4	28.6	30.76	1	0.79	-49.1	0.05	0.82	0.15	0.16	0.8	30.26	30.09	29.29	29.3	6.72	14.9		
1 7 to 1 8	RJRJ	26	60.0	0.283	7.5	13.3	8.38	130.75	1.4818	53.8	7.54	136.68	1.4336	544.3	544.3	54.3	54.3	189.1	0.29	1.98	5.82	2.24	6.69	30.76	28.55	26.64	26.64	1	1.44	0	0.05	0.76	0.77	0.19	0.14	1.96	29.3	29.15	27.19	27.19	7.55	13.2	
1 8 to 1 9	RJRJ	15.66	60.0	0.283	7.5	13.3	8.59	129.45	1.4818	53.8	7.76	134.95	1.4336	537.4	537.4	53.7	53.7	189.1	0.28	1.9	5.8	2.22	6.69	28.64	26.55	25.38	25.38	25.66	1	0.98	51.3	0.909	0.2	0.18	0.04	0.03	27.19	27.15	25.59	25.58	9.94	10.1	
1 9 to 1 10	RJRJ	7.3	7.50	0.442	4	25	8.72	128.67	2.077	74.2	7.75	135.03	2.0156	756	756	75.6	75.6	247.75	0.31	1.71	4.96	2.22	5.61	26.96	24.47	24.18	26.07	1	1.12	0	1.67	1.9	0.15	0.25	0.28	0.03	25.58	25.29	25.27	25.27	0.58	26.53	
2 1 to 2 2	RJRJ	14.69	37.5	0.111	4.95	20.2	6	149	0.0109	4.5	6	149	0.0109	4.5	4.5	4.5	4.466	0.01	0.04	1.33	0.56	4.04	42.53	41.07	40.34	42	1	1	48.8	0.05	4.5	0	0.077	41.44	41.44	40.67	40.67	5.27	19.9				
2 2 to 2 3	RJRJ	22.77	37.5	0.111	1.7	58.9	6.12	147.78	0.067	27.5	6	149	0.0668	27.6	27.6	2.1	28.7	161	0.11	0.26	1.57	0.99	2.36	42	40.29	39.91	41.43	1	1.12	-10.3	0.05	0.65	0	0	0.43	40.67	40.67	40.24	40.24	1.68	59.2		
2 3 to 2 4	RJRJ	24.03	37.5	0.111	2.75	36.3	6.31	145.88	0.0931	37.7	6.19	147.1	0.0929	37.9	3.5	39.5	33.27	0.12	0.36	2.05	1.02	3.01	41.43	38.86	39.19	40.71	1	1.12	10.7	0.05	1.44	0.01	0.01	0.7	40.24	40.23	39.55	39.53	2.91	34.4			
2 4 to 2 5	RJRJ	26.35	37.5	0.111	1	100	6.51	145.95	0.1201	48	6.39	145.91	0.1199	48.3	48.3	1.5	49.8	199.5	0.25	0.45	1.51	1.1	1.81	40.71	39.14	38.58	40.22	1	1	-54.8	1.459	1.22	0.01	0.01	39.55	39.52	39.01	39.01	5.15	51.7			
2 5 to 2 6	RJRJ	20.46	37.5	0.111	16.9	59.1	6	149	0.0878	36.3	6	149	0.0878	36.3	3.6	3.6	-2.4	32.4	260.4	0.13	0.31	1.65	0.98	2.36	42.7	41.24	40.89	42.48	1.2	1.61	6.1	0.05	4.5	0	0.02	0.36	41.64	41.62	41.25	41.25	1.78	36.3	
2 6 to 2 7	RJRJ	25.06	37.5	0.111	1	100	6.19	147.12	0.1963	80.2	6	147.1	0.1961	80.2	80.2	-1.5	78.7	199.9	0.39	0.71	1.71	1.27	1.81	42.48	40.84	40.59	42.16	1	1.16	-28.6	0.15	1.21	0.03	0.03	23.1	41.25	41.22	40.99	40.99	1.11	50.5		
2 7 to 2 8	RJRJ	6.44	52.5	0.216	1.57	63.6	6.4	145.04	0.3384	15.5	6.4	145.2	0.3381	15.4	15.4	15.5	15.5	168.04	0.25	0.71	2.36	1.42	2.81	41.26	40.44	40.19	41.78	1	1.19	30	0.05	0.98	0.03	0.02	0.29	40.49	40.97	40.68	40.68	1.76	56.7		
2 8 to 2 9	RJRJ	18.42	52.5	0.216	3.39	29.5	6.53	143.8	0.4061	162.2	6	143.9	0.4059	162.2	162.2	-1.5	160.7	894.3	0.18	0.74	3.16	1.44	4.13	41.78	40.14	39.52	41.28	1	1.2	0	0.05	0.66	0	0.02	0.65	40.68	40.66	40.41	40.41	3.57	28		
2 9 to 2 10	RJRJ	21	52.5	0.216	14.9	66.9	6.68	142.8	0.4367	173.3	6.6	142.9	0.4365	173.4	173.4	1.5	171.9	59.9	0.29	0.79	2.39	1.48	2.74	41.29	39.47	39.16	40.77	1	1.06	0.1	0.05	0.55	0	0.02	0.34	40.01	39.99	39.65	39.65	1.63	61.3		
2 10 to 2 11	RJRJ	23.52	52.5	0.216	1	100	6.86	141.84	0.4728	186.3	6.64	141.94	0.4726	186.3	186.3	-1.5	184.8	484.8	0.38	0.85	2.09	1.51	2.24	40.77	39.11	38.87	40.36	1	1	-0.1	0.466	0.58	0.04	0.02	0.38	39.65	39.12	39.12	39.12	2.71	46.1		
2 11 to 2 12	RJRJ	21.22	52.5	0.216	1	100	6	149	0.0788	32.6	6	149	0.0788	32.6	32.6	3.1	31.1	199.9	0.16	0.28	1.31	0.95	1.81	42.5	41.12	40.89	42.48	1	1	-54.3	0.05	4.5	0	0.02	0.24	41.51	41.49	41.25	41.25	1.08	92.3		
2 12 to 2 13	RJRJ	5	45.0	0.159	11	9.1	6	149	0.0592	246.3	6	149	0.0592	246.3	246.3	-2.4	246.3	1076.8	0.23	0.55	1.55	1.86	6.77	27.92	25.52	24.77	25.58	1	1.8	-38.7	0.3	4.5	0.12	0.55	0.22	26.32	25.77	25.54	25.54	4.47	22.4		
2 13 to 2 14	RJRJ	20.43	37.5	0.111	10.85	9.2	6.17	147.29	0.0387	15.6	6	149	0.0387	15.6	15.6	1.5	156.62	0.02	0.14	2.56	0.79	6	43.61	42.15	39.93	42.08	1.2	1.42	-45.1	0.051	0.5	0.1	0.02	2.07	42.52	42.50	42.45	42.45	10.92	12.9			
2 14 to 2 15	RJRJ	69.09	45.0	0.159	12.8	7.8	6.87	141.79	0.5774	227.4	6.67	142.95	0.5703	226.4	227.4	2.1	227.4	1162	0.02	0.14	1.43	5.72	1.79	7.31	42.08	39.88	31.35	32.03	1	1.03	38.5	0.05	1.15	0.11	0.02	1.91	31.54	31.49	29.52	29.52	15.19	7.6	
2 15 to 2 16	RJRJ	14.52	52.5	0.216	13.4	59.3	6.67	147.44	0.6331	241.7	7.25	149.99	0.6329	241.7	241.7	-1.5	241.7	1217.7	0.02	0.14	1.43	6.63	1.24	7.63	43.25	40.98	38.96	40.88	1	1	4	0.05	0.98	0.16	0.16	0.08	0.08	41.54	41.52	41.47	41.47	3.58	11.6
2 16 to 2 17	RJRJ	14.88	52.5	0.216	2.93	34.2	6.69	144.85	0.4645	166.9	6.62	147.69	0.4574	186.5	186.9	-1.5	186.9	323	0.58	0.18	2.1	1.63	2.03	42.99	40.62	40.52	45.15	1	1	9	0.05	1.64	0.07	0.02	0.09	0.09	0.09	41.42	41.07	40.98	40.98	1.05	11.65
2 17 to 2 18	RJRJ	18.75	52.5	0.216	2.93	34.2	6.69	148.8	0.1561	201.6	6.62	148.9	0.1561	201.6	201.6	-1.5	195.7	83.1	0.72	0.75	2.75	1.84	3.84	37.53	34.76	34.76	36.33	1	1	9.8	0.05	0.51	0.39	0.2	0.28	36.12	35.92	35.64	35.64	1.51	66.3		
2 18 to 2 19	RJRJ	18.75	52.5	0.216	2.93	34.2	6.69	148.8	0.1561	201.6	6.62	148.9	0.1561	201.6	201.6	-1.5	195.7	83.1	0.72	0.75	2.75	1.84	3.84	37.53	34.76	34.76	36.33	1	1	9.8	0.05	0.51	0.39	0.2	0.28	36.12	35.92	35.64	35.64	1.51	66.3		
2 19 to 2 20	RJRJ	22.99	52.5	0.216	1	100	7.26	138.91	0.1077	415.6	6.73	100.74	0.1069	417.5	417.5	-1.5	417.5	485.6	0.86	0.29	2.5	2.18	2.24	3.55	33.27	33.05	34.92	34.92	1	1.46	26.62	0.075	0.65	0.19	0.12	0.16	34.38	33.71	33.71	33.71	7.04	134.4	
2 20 to 2 21	RJRJ	14.02	50.0	0.159	14.02	50.0	6	149	0.0355	56.1	6	149	0.0355	56.1	56.1	-1.5	56.1	96.1	0.44	0.12	0.51	2.81	1.14	4.07	42.67	40.95	39.75	41.09	1.2	1	7.21	0.05	4.5</td										

2D MODEL - 100yr OVERLAND FLOW

100 YEAR HYDRAULIC DATA

it	Bypass Pit	Choke	Intensity (mm/h)	Base Inflow (Cumecs)	Catchment Flow (Cumecs)	Inlet Flow (Cumecs)	Pipe Flow (Cumecs)	Bypass Flow (Cumecs)	Flooded Depth (m)	Flooded Velocity (m/s)	Max V.d. (m.m/s)	Flooded Width (m)
1	1\2	0.8	178	0	0.401859	0.321487	0.321487	0.080372				
2	1\3	0.5	178	0	0.037323	0.122834	0.666863	0	0.113496			
3	LOST	0.5	178	0	0.051136	0.055349	0.788195	0	0.069865			
4		0.8		0			0.782019					
5		0.8		0			0.770656					
6		0.8		0			0.757927					
7		0.8		0			0.755247					
8		0.8		0			0.749394					
9		0.8		0			1.052737					
1	2\2	0.8	178	0	0.005995	0.004796	0.004796	0.001199				
2	1\3	0.8	178	0	0.030958	0.031775	0.036495	0.012508				
3	2\4	0.8	178	0	0.014393	0.02102	0.057135	0.005881	0.065193	0.853664	0.055653	1.311172
4	1\3	0.8	178	0	0.014891	0.01656	0.073063	0.004213	0.060491	0.811931	0.049114	1.145121
1	3\2	0.8	178	0	0.048421	0.034038	0.034038	0.014383	0.08853	0.683191	0.060483	2.085374
2	3\3	0.8	178	0	0.016427	0.023774	0.088327	0.007037	0.072125	0.582101	0.041984	2.268699
3	3\4	0.8	178	0	0.025441	0.024801	0.188377	0.007677	0.070676	0.700013	0.049474	1.981713
4	3\5	0.8	178	0	0.012539	0.016155	0.203142	0.004961	0.060345	0.811485	0.048969	1.094314
5	3\6	0.8	178	0	0.016887	0.016688	0.218129	0.004261	0.060642	0.813268	0.049318	1.150192
6	1\2	0.8	178	0	0.019912	0.019034	0.234991	0.005139	0.063267	0.836615	0.05293	1.238228
1	2\2	0.8	178	0	0.004349	0.031314	0.031314	0.012125	0.0858	0.666334	0.057171	1.98992
2		0.8	178	0	0.328296	0.328296	0.328296	0				
3	7\1	0.8	178	0	0.00925	0.0074	0.0074	0.001885	0.053973	0.490415	0.026469	0.929214
4	6\3	0.8	178	0	0.012105	0.009684	0.016941	0.002421	0.054313	0.63477	0.034476	0.931887
5	6\4	0.8	178	0	0.012591	0.01201	0.0304723	0.003902	0.042845	1.416395	0.060685	0.5534
6	LOST	0.8	178	0	0.030752	0.026751	0.323293	0.007004	0.054372	1.73316	0.094235	0.959345
7	LOST	0.8	178	0	0.065584	0.05031	0.368787	0.015274	0.070661	1.980858	0.139969	1.221322
8	7\2	0.5	178	0	0.121746	0.123596	0.123595	0	0.126009			
9	7\3	0.5	178	0	0.06339	0.023702	0.145885	0.039688	0.047467			
10		0.8		0			0.145095					
11	7\4	0.5	178	0	0.071078	0.110766	0.253879	0	0.11782			
12	7\5	0.5	178	0	0.005165	0.005165	0.25800	0	0.040731			
13	6\5	0.5	178	0	0.023153	0.023153	0.279916	0	0.046834			
14		0.8		0			0.27856					
15	8\1	0.8	178	0	0.425404	0.340323	0.340323	0.085081				
16	8\3	0.8	178	0	0.025454	0.061962	0.522858	0.048573	0.08907	1.535091	0.13673	2.103471
17	8\4	0.8	178	0	0.027273	0.069873	0.629603	0.061229	0.077706	1.565306	0.121634	3.009705
18	8\5	0.8	178	0	0.065577	0.069316	0.693246	0.05749	0.087967	1.797556	0.158125	2.093741
19	8\6	0.8	178	0	0.050818	0.061157	0.747893	0.047151	0.088047	1.550102	0.136482	2.069172
20	8\7	0.8	178	0	0.048396	0.055172	0.796328	0.040375	0.096883	1.067983	0.10347	2.365491
21	LOST	0.5	178	0	0.090088	0.151219	0.94047	0.080133	0.129288			
22	LOST	0.5	178	0	0.117126	0.135974	1.303906	0	0.121106			
23	8\8	0.8	178	0	0.061173	0.042415	0.042415	0.018757	0.072263	1.580748	0.11423	1.423134
24	8\3	0.5	178	0	0.058604	0.040395	0.040395	0.018254	0.072925	1.372947	0.100122	1.574001
25	8\1	0.8	178	0	0.047248	0.02619	0.125468	0.036002	0.050292			
26	8\2	0.8	178	0	0.074718	0.059774	0.059774	0.014944				
27	12\2	0.8	178	0	0.079669	0.063735	0.063735	0.015934				
28	12\4	0.8	178	0	0.058452	0.046616	0.109773	0.02777	0.044963	6.217968	0.279577	0.53213
29	12\4	0.8	178	0	0.0554	0.043525	0.191223	0.021716	0.021205	3.024824	0.036449	2.947025
30	12\5	0.8	178	0	0.04031	0.042829	0.230804	0.019197	0.071669	1.532263	0.109815	1.519957
31	12\6	0.8	178	0	0.040869	0.041889	0.382967	0.018197	0.071011	1.521647	0.108054	1.49791
32	12\7	0.5	178	0	0.049497	0.070923	0.497121	0	0.089817			
33	12\8	0.8	178	0	0.025212	0.040409	0.533328	0.020707				
34	12\9	0.8	178	0	0.017016	0.028326	0.557752	0.009397	0.068288	0.909028	0.062076	1.82918
35	12\10	0.8	178	0	0.017296	0.026559	0.669928	0.008874	0.025274	4.359154	0.110174	0.640139
36	LOST	0.8	178	0	0.00959	0.014689	0.681184	0.036674	0.064912	0.595891	0.038681	1.285258
37	12\3	0.8	178	0	0.049205	0.039364	0.039364	0.009841				
38	12\4	0.8	178	0	0.076426	0.061141	0.061141	0.015285				
39	12\7	0.8	178	0	0.049171	0.056322	0.116937	0.035904	0.080418	1.674945	0.134696	1.813362
40	15\2	0.8	178	0	0.016322	0.013057	0.013057	0.003264				
41	15\3	0.8	178	0	0.027266	0.023601	0.036495	0.006929	0.063236	0.600086	0.037961	3.134611
42	12\6	0.8	178	0	0.009208	0.012908	0.049205	0.003228	0.05573	0.521042	0.029038	2.354672
43	16\2	0.8	178	0	0.00724	0.006792	0.006792	0.001448				
44	16\3	0.8	178	0	0.038567	0.029441	0.035152	0.010573				
45	LOST	0.8	178	0	0.022145	0.024949	0.053939	0.007679	0.075521	0.702209	0.053031	1.655276
46	12\9	0.8	178	0	0.043201	0.034561	0.034561	0.008664				
47	18\2	0.8	178	0	0.051198	0.040334	0.040334	0.010863				
48	18\3	0.8	178	0	0.051707	0.076696	0.11551	0.025874				
49	18\4	0.8	178	0	0.05088	0.060374	0.173993	0.016381				
50	18\7	0.8	178	0	0.082622	0.070114	0.240797	0.028889				
51		0.8		0	0.022201	0.022201	0.022201	0.002201				

FOR CONSTRUCTION CERTIFICATE



client:
HAMMERSMITH MANAGEMENT PTY LTD

project: **CAMERON GROVE - LINK ROAD
ROAD & DRAINAGE DESIGN**

PROJECT NUMBER: 111111111111111111111111

Revisions	00	AW	DD	CB	TT	12/04/13	ISSUED FOR 80% REVIEW	 CONSULT AUSTRALIA Member Firm	<p>Disclaimer and Copyright: ALL DIMENSIONS TO BE CHECKED ON SITE BY SUPERINTENDENT PRIOR TO CONSTRUCTION. USE WRITTEN DIMENSIONS ONLY, DO NOT SCALE.</p> <p>BY: STUART GREEN FIEAust, CPEng, Manager - Urban Development SIGN: </p> <p>DATE: </p>	Hammersmith Management					
	01	AW	DD	CB	SG	2/10/13	ISSUED FOR CONSTRUCTION CERTIFICATE			Cameron Grove - Link R					
02	VP	JP	SG			16/07/14	ISSUED WITH COUNCIL AMENDMENTS			Road & Drainage Design					
03	BL	BL	PF	SAG		25/05/15	UPDATED TO COUNCIL COMMENTS								
04	BL	BL	PF	SAG		5/08/15	NOTE ADDED TO RIGID PAVEMENT PLAN REGARDING SURFACE FINISH								
05	BL	BL	PF			02/09/15	UPDATED TO REFLECT COMMENTS BY GEOTECHNICAL ENGINEER								
First	AW	DD	CB	TT		12/04/13									
Issue	Drawn	Design	Check	Appd.	Date		Revision Details								